



Designation: ~~D6270~~—17 D6270 – 20

Standard Practice for Use of Scrap Tires in Civil Engineering Applications¹

This standard is issued under the fixed designation D6270; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ϵ) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 This practice provides guidance for testing the physical properties, design considerations, construction practices, and leachate generation potential of processed or whole scrap tires in lieu of conventional civil engineering materials, such as stone, gravel, soil, sand, lightweight aggregate, or other fill materials.

1.2 The values stated in SI units are to be regarded as standard. No other units of measurement are included in this standard.

1.3 *This international standard was developed in accordance with internationally recognized principles on standardization established in the Decision on Principles for the Development of International Standards, Guides and Recommendations issued by the World Trade Organization Technical Barriers to Trade (TBT) Committee.*

2. Referenced Documents

2.1 ASTM Standards:²

~~C127~~ **C127** Test Method for Relative Density (Specific Gravity) and Absorption of Coarse Aggregate

~~C136~~ **C136** Test Method for Sieve Analysis of Fine and Coarse Aggregates

D698 Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³ (600 kN-m/m³))

D1557 Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³))

D1566 Terminology Relating to Rubber

D2434 Test Method for Permeability of Granular Soils (Constant Head)

D2974 Test Methods for Determining the Water (Moisture) Content, Ash Content, and Organic Material of Peat and Other Organic Soils

~~D3080~~ **D3080** Test Method for Direct Shear Test of Soils Under Consolidated Drained Conditions (Withdrawn 2020)³

D4253 Test Methods for Maximum Index Density and Unit Weight of Soils Using a Vibratory Table

D5681 Terminology for Waste and Waste Management

D7760 Test Method for Measurement of Hydraulic Conductivity of Materials Derived from Scrap Tires Using a Rigid Wall Permeameter

F538 Terminology Relating to the Characteristics and Performance of Tires

¹ This practice is under the jurisdiction of ASTM Committee D34 on Waste Management and is the direct responsibility of Subcommittee D34.03 on Treatment, Recovery and Reuse.

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² For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

³ The last approved version of this historical standard is referenced on www.astm.org.

2.2 *American Association of State Highway and Transportation Officials Standards:*

T 274 Standard Method of Test for Resilient Modulus of Subgrade Soils⁴

M 288 Standard Specification for Geotextiles⁵

2.3 *U.S. Environmental Protection Agency Standard:*

Method 1311 Toxicity Characteristics Leaching Procedure⁶

3. Terminology

3.1 *Definitions*—For definitions of common terms used in this practice, refer to Terminologies **D5681** (waste management), **F538** (tires), and **D1566** (rubber), respectively.

3.2 *Definitions of Terms Specific to This Standard:*

3.2.1 *bead wire, n*—a high-tensile steel wire surrounded by rubber, which forms the bead of a tire that provides a firm contact to the rim.

3.2.2 *casing, n*—the tire structure not including the tread portion of the tire.

3.2.3 *mineral soil, n*—soil containing less than 5 % organic matter as determined by a loss on ignition test—~~test~~. **(D2974)**

3.2.4 *preliminary remediation goal, n*—risk-based concentrations that the USEPA considers to be protective for lifetime exposure to humans.

3.2.5 *rough shred, n*—a piece of a shredded tire that is larger than 50 by 50 by 50 mm, but smaller than 762 by 50 by 100 mm.

3.2.6 *rubber buffings, n*—vulcanized rubber usually obtained from a worn or used tire in the process of removing the old tread in preparation for retreading.

3.2.7 *rubber fines, n*—small particles of ground rubber that result as a by-product of producing shredded rubber.

3.2.8 *scrap tire, n*—a pneumatic rubber tire discarded because it no longer has value as a new tire, but can be either reused and processed for similar applications as new or processed for other applications not associated with its originally intended use.

3.2.9 *steel belt, n*—rubber-coated steel cords that run diagonally under the tread of steel radial tires and extend across the tire approximately the width of the tread.

3.2.10 *tire chips, n*—pieces of scrap tires that have a basic geometrical shape and are generally between 12 and 50 mm in size and have most of the wire removed.

3.2.11 *tire-derived aggregate (TDA), n*—pieces of scrap tires that have a basic geometrical shape and are generally between 12 and 305 mm in size and are intended for use in civil engineering applications.

3.2.12 *waste tire, n*—a tire that is no longer capable of being used for its original purpose, but has been disposed of in such a manner that it cannot be used for any other purpose.

3.2.13 *whole tire, n*—a tire that has been removed from a rim but has not been processed.

4. Significance and Use

4.1 This practice is intended for use of scrap tires including: ~~tire-derived~~ tire-derived aggregate (TDA) comprised of pieces of

⁴ *Standard Specifications for Transportation Materials and Methods of Sampling and Testing, Part II: Methods of Sampling and Testing*, American Association of State Highway and Transportation Officials, Washington, DC.

⁵ *Standard Specifications for Transportation Materials and Methods of Sampling and Testing, Part I: Specifications*, American Association of State Highway and Transportation Officials, Washington, DC.

⁶ *Test Methods for Evaluating Solid Waste: Physical/Chemical Methods*, 3rd ed., Report No. EPA 530/SW-846, U.S. Environmental Protection Agency, Washington, DC.

scrap tires, TDA/soil mixtures, tire sidewalls, and whole scrap tires in civil engineering applications. This includes use of TDA and TDA/soil mixtures as lightweight embankment ~~fill~~, fill; lightweight retaining wall ~~backfill~~, backfill; drainage layers for roads, landfills, and other ~~applications~~, applications; thermal insulation to limit frost penetration beneath ~~roads~~, roads; insulating backfill to limit heat loss from ~~buildings~~, buildings; vibration damping layers for rail ~~lines~~, lines; and replacement for soil or rock in other fill applications. Use of whole scrap tires and tire sidewalls includes construction of retaining walls, drainage culverts, road-base reinforcement, and erosion protection, as well as use as fill when whole tires have been compressed into bales. It is the responsibility of the design engineer to determine the appropriateness of using scrap tires in a particular application and to select applicable tests and specifications to facilitate construction and environmental protection. This practice is intended to encourage wider utilization of scrap tires in civil engineering applications.

4.2 Three TDA fills with thicknesses in excess of 7 m have experienced a serious heating reaction. However, more than 100 fills with a thickness less than 3 m have been constructed with no evidence of a deleterious heating reaction **(1)**.⁷ Guidelines have been developed to minimize internal heating of TDA fills **(2)** as discussed in 6.11. The guidelines are applicable to fills less than 3 m thick. Thus, this practice should be applied only to TDA fills less than 3 m thick.

5. Material Characterization

5.1 The specific gravity and water absorption capacity of TDA should be determined in accordance with Test Method **C127**. However, the specific gravity of TDA is less than half the value obtained for common earthen coarse aggregate, so it is permissible to use a minimum weight of test sample that is half of the specified value. The particle density or density of solids of TDA (ρ_s) may be determined from the apparent specific gravity using the following equation:

$$\rho_s = S_a(\rho_w) \quad (1)$$

where:

S_a = apparent specific gravity, and

ρ_w = density of water.

5.2 The gradation of TDA should be determined in accordance with Test Method ~~C136~~ **C136/C136M**. However, the specific gravity of TDA is less than half the values obtained for common earthen materials, so it is permissible to use a minimum weight of test sample that is half of the specified value.

5.3 The laboratory-compacted dry density (or bulk density) of TDA and TDA/soil mixtures with less than 30 % retained on the 19.0-mm sieve can be determined in accordance with Test Method ~~D698~~ **D698** or **D1557**. However, TDA and TDA/soil mixtures used for civil engineering applications almost always have more than 30 % retained on the 19.0-mm sieve, so these methods generally are not applicable. A larger compaction mold should be used to accommodate the larger size of the TDA. The sizes of typical compaction molds are summarized in **Table 1**. The larger mold requires that the number of layers, or the number of blows of the rammer per layer, or both, be increased to produce the desired compactive energy per unit volume. Compactive energies ranging from 60 % of Test Method ~~D698~~ **D698** (60 % \times 600 kN-m/m³ = 360 kN-m/m³) to 100 % of Test Method ~~D1557~~ **D1557** (2700 kN-m/m³) have been used. Compaction energy has only a small effect on the resulting dry density **(3)**; thus, for most applications it is permissible to use a compactive energy equivalent to 60 % of Test Method ~~D698~~ **D698**. To achieve this energy with a mold volume of 0.0125 m³ would require that the sample be compacted in five layers with 44 blows per layer with a 44.5 N rammer falling 457 mm. The water content of the sample has only a small effect on the compacted dry density **(3)** so it is permissible to perform compaction tests on air or oven-dried samples.

TABLE 1 Size of Compaction Molds Used to Determine Dry Density of TDA

Maximum Particle Size (mm)	Mold Diameter (mm)	Mold Volume (m ³)	Reference
75	254	0.0125	(3)
75	305	0.0146	(4)
51	203 and 305	N.R. ^A	(5)

^A N.R. = not reported.

⁷ The boldface numbers in parentheses refer to the list of references at the end of this standard.

5.3.1 The dry densities for TDA loosely dumped into a compaction mold and TDA compacted by vibratory methods (similar to Test Method [Methods D4253](#)) are about the same (4-6). Thus, vibratory compaction of TDA in the laboratory (see Test Method [Methods D4253](#)) should not be used.

5.3.2 When estimating an in-place density for use in design, the compression of a TDA layer under its own self-weight and under the weight of any overlying material must be considered. The dry density determined as discussed in 5.3 are uncompressed values. In addition, short-term time-dependent settlement of TDA should be accounted for when estimating the final in-place density (7).

5.3.3 Values of the secant constrained modulus, M_{sec} , which vary linearly with the compacted unit weight and applied vertical stress, can be estimated as (8):

$$M_{sec} = 1.8\sigma_v + 115\gamma - 458 \text{ kPa} \quad (2)$$

where:

σ_v = vertical stress, and
 γ = compacted unit weight, kN/m^3 .

5.3.4 Time-dependent settlement for an average duration of four weeks, ΔH_t , can be calculated as (9):

$$\Delta H_t = HC_{ac} \log \frac{t_1}{t_2} \quad (3)$$

where:

C_{ac} = modified secondary compression index ≈ 0.0065 for 100 % TDA,
 H = thickness of the TDA layer,
 t_1 = time when time-dependent compression begins (assumed to be one day), and
 t_2 = time at which the magnitude of time-dependent compression is required.

For long-term settlement, refer to [X1.11](#).

5.4 The compressibility of TDA and TDA/soil mixtures can be measured by placing TDA in a rigid cylinder with a diameter several times greater than the largest particle size and then measuring the vertical strain caused by an increasing vertical stress. If it is desired to calculate the coefficient of lateral earth pressure at rest $K_{\theta\theta}$, the cylinder can be instrumented to measure the horizontal stress of the TDA acting on the wall of the cylinder.

5.4.1 The high compressibility of TDA necessitates the use of a relatively thick sample. In general, the ratio of the initial specimen thickness to sample diameter should be greater than one. This leads to concerns that a significant portion of the applied vertical stress could be transferred to the walls of the cylinder by friction. If the stress transferred to the walls of the cylinder is not accounted for, the compressibility of the TDA will be underestimated. For all compressibility tests, the inside of the container should be lubricated to reduce the portion of the applied load that is transmitted by side friction from the sample to the walls of the cylinder. For testing where a high level of accuracy is desired, the vertical stress at the top and the bottom of the sample should be measured so that the average vertical stress in the sample can be computed. A test apparatus designed for this purpose is illustrated in [Fig. 1 \(10\)](#).

5.5 The resilient modulus (M_R) of subgrade soils can be expressed as:

$$M_R = A\theta^B \quad (4)$$

where:

θ = first invariant of stress (sum of the three principal stresses),
 A = experimentally determined parameter, and
 B = experimentally determined parameter.

5.5.1 Tests for the parameters A and B can be conducted according to AASHTO T 274. The maximum particle size typically is limited to 19 mm by the testing apparatus, which precludes the general applicability of this procedure to the larger size TDA typically used for civil engineering applications.

supply of water and measuring the head loss across the sample using standpipes mounted on the body of the permeameter. An apparatus that takes these factors into account is shown in Fig. 2 (11). A standard test method for measurement of hydraulic conductivity of TDA is provided in Test Method D7760.

5.9 The thermal conductivity of TDA is significantly lower than for common soils. For TDA smaller than 25 mm in size, the thermal conductivity can be measured using commercially available guarded hot plate apparatus. For TDA larger than 25 mm, it is necessary to construct a large-scale hot plate apparatus (15). The thermal conductivity of TDA also can be back-calculated from field measurements (15).

6. Construction Practices

6.1 TDA have a compacted dry density that is one-third to one-half one third to one half of the compacted dry density of typical soil. This makes them an attractive lightweight fill for embankments constructed on weak, compressible soils where slope stability or excessive settlement are a concern, as well as landslide repair.

6.2 The thermal resistivity of TDA is approximately eight times greater than for typical granular soil. For this reason, TDA can be used as a 150 to 450-mm thick insulating layer to limit the depth of frost penetration beneath roads. This reduces frost heave in the winter and improves subgrade support during the spring thaw. In addition, TDA can be used as backfill around basements to limit heat lost through basement walls, thereby reducing heating costs.

6.3 The low compacted dry density, high hydraulic conductivity, and low thermal conductivity make TDA very attractive for use as retaining wall backfill. Lateral earth pressures for TDA backfill can be about 50 % of values obtained for soil backfill (7-127, 10, 12). TDA can also be used as backfill for geosynthetic-reinforced retaining walls. An at-rest value of $K_0 = 0.3$ has been recommended for the design of cantilever retaining walls with TDA backfill up to 3 m thick (8, 16-18).

6.4 The hydraulic conductivity of TDA makes them suitable for many drainage applications including French drains, drainage layers in landfill liner and cover systems, and leach fields for on-site sewage disposal systems. For applications with a vertical stress less than 50 kPa, the hydraulic conductivity of TDA is generally greater than 1 cm/s, which is comparable to conventional uniformly graded aggregate. When TDA is used as a component of landfill leachate collection and removal systems, and other applications where the vertical stress would be greater than 50 kPa, the hydraulic conductivity and void ratio under the final design vertical stress should be considered. The hydraulic conductivity must meet applicable regulatory requirements and the void ratio must be sufficient to minimize clogging.

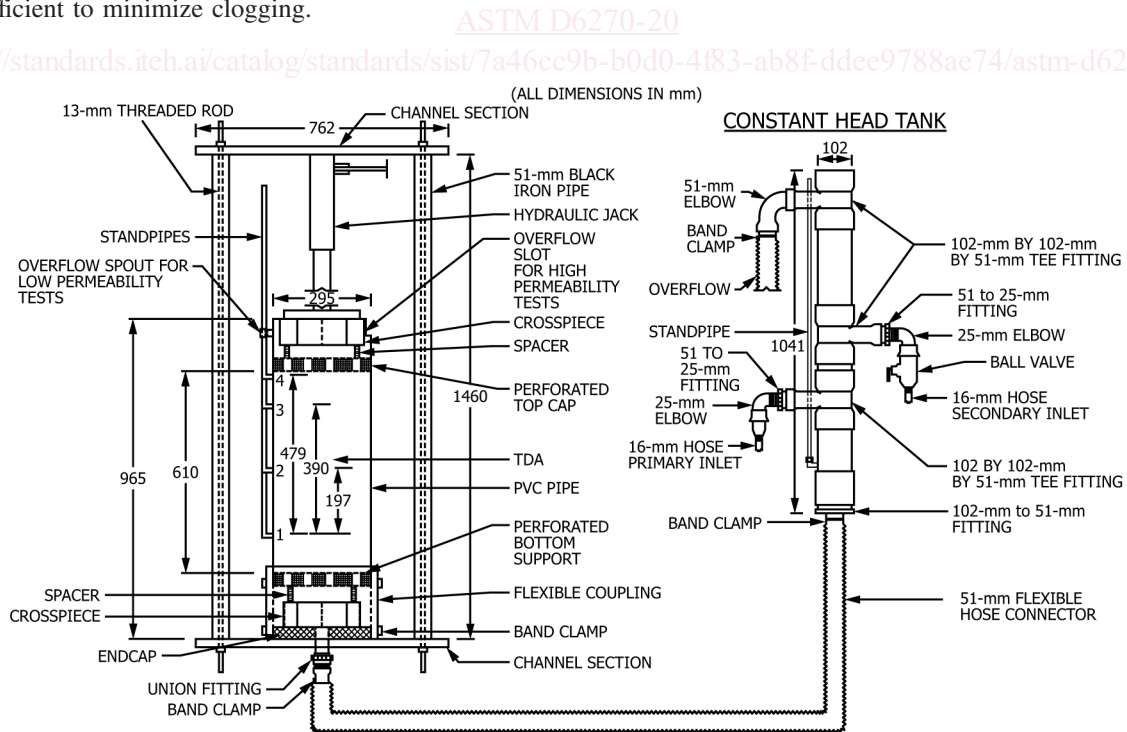


FIG. 2 Hydraulic Conductivity Apparatus for TDA with Provisions for Application of Vertical Stress (14)

6.4.1 TDA can be used as a substitute for gravel in landfill horizontal gas collection trenches. In this application, 152 mm of TDA is placed on the bottom of the trench as a base material for the gas collection pipe. After the pipe is in place, an additional 305 mm of TDA is placed over the pipe (19).

6.5 TDA can be used as a vibration damping layer beneath rail lines to reduce the impact of ~~ground-bourn~~ground-borne vibrations above 16 Hz on residences and businesses adjoining the tracks. In this application, a 300-mm thick layer of 75-mm maximum size TDA wrapped in filter fabric is placed beneath the conventional ballast/subballast system (20-23).

6.6 Two different sizes of TDA are commonly used for the applications discussed above. One has a maximum size of 75 mm and the other has a maximum size of 300 mm. Rough shreds can also be used for some applications, provided all tires are shredded such that the largest shred is the lesser of ~~one-quarter~~one quarter circle in shape or 600 mm in length. In all cases, at least one side wall should be severed from the tread.

6.7 TDA with a maximum size of 75 mm or 300 mm are generally placed in 300-mm thick lifts and compacted by a tracked bulldozer, sheepsfoot roller, or smooth drum vibratory roller with a minimum operating weight of 90 kN. Rough shreds are generally placed in 900-mm thick lifts and compacted by a tracked bulldozer. For most applications, a minimum of six passes of the compaction equipment should be used.

6.8 TDA should be covered with a sufficient thickness of soil to limit deflections of overlying pavement caused by traffic loading. Soil cover thicknesses as low as 0.8 m may be suitable for paved roads with light traffic. For paved roads with heavy traffic, 1 to 2 m of soil cover may be required. For unpaved applications, 0.3 to 0.5 m of soil cover may be suitable depending on the traffic loading. The designer should assess the actual thickness of soil cover needed based on the loading conditions, TDA layer thickness, pavement thickness, and other conditions as appropriate for a particular project. Regardless of the application, the TDA should be covered ~~with soil in such a way as~~ to prevent contact between the public and the TDA, which may have exposed steel belts.

6.9 In applications where pavement will be placed over the TDA layer, highway drainage applications, and retaining wall backfill, the TDA layer should be completely wrapped in a layer of ~~non-woven or woven~~ geotextile to minimize infiltration of soil particles into the voids between the TDA. AASHTO M 288 should be used for guidance on geotextile selection.

6.10 Whole scrap tires and tire sidewalls that have been cut from the tire casing can be used to construct retaining walls, reinforcing mats beneath roads constructed on weak ground, and erosion protection layers.

6.11 TDA fills should be designed to minimize the possibility of an internal heating reaction (2). ~~Possible causes~~Oxidation of the reaction are ~~oxidation of the exposed steel belts and oxidation of the rubber~~. Microbes may play a role in ~~both~~exposed wire is the primary mechanism for an exothermic reaction responsible for self-heating in TDA ~~reactions~~reactions(24). ~~Factors thought to create eonditions~~Conditions favorable for oxidation of ~~exposed steel~~steel or rubber, or both, include: ~~free access to air; free access to water; retention of heat caused by the high insulating value of TDA in combination with a large fill thickness; large amounts of exposed steel belts; and smaller TDA sizes and excessive amounts of granulated rubber particles; and the presence of inorganic and organic nutrients that would enhance microbial action~~particles.

6.11.1 ~~The design guidelines given in the following sections were developed to minimize the possibility for heating of TDA fills by minimizing factors that could create conditions favorable for this reaction. In developing these guidelines, the insulating effect caused by increasing fill thickness and the favorable performance of projects with TDA fills less than 4-m thick have been considered. Thus, design guidelines are less stringent for projects with thinner TDA layers. The guidelines are divided into two classes: Class I fills with TDA layers less than 1-m thick, and Class II fills with TDA layers in the range of 1 to 3-m thick. Although there have been no projects with less than 4 m of TDA fill that have experienced a catastrophic heating reaction, to be conservative, TDA layers greater than 3-m thick are not recommended. The guidelines are for use in designing TDA fills. TDA layers of greater than 3 m vertical thickness are not recommended. A 3-m TDA fill which is constructed based on current design guidelines should not experience an exothermic reaction resulting in self-heating that leads to combustion (24). Design of fills that are mixtures or alternating layers of TDA and mineral soil should be handled on a case-by-case basis.~~

6.11.2 ~~For Class I fills, the material shall meet the material requirements for Type A TDA given in 7.1.1 and 7.1.2. No special design features are required to minimize heating of Class I fills.~~

6.11.3 ~~For Class II fills, the material shall meet the material requirements for Type B TDA given in 7.1.1 and 7.1.3.~~

6.11.2 ~~Class II fills~~ Fills shall be constructed in such a way that infiltration of water and air is minimized. Moreover, there shall be no direct contact between TDA and soil containing organic matter, such as topsoil, organic matter. One possible way to accomplish this is to cover the top and sides of the fill with a 0.5-m thick layer of compacted mineral soil with a minimum of 30 % rubber fines. The mineral soil. The soil should be separated from the TDA with a geotextile. The top of the mineral soil layer should be sloped so that water will drain away from the TDA fill. geotextile fabric. Additional fill may be placed on top of the mineral soil layer as needed to meet the overall design of the project. If the project will be paved, it is recommended that the pavement extend to the shoulder of the embankment or that other measures be taken to minimize infiltration at the edge of the ~~There is no need to try to exclude water or air movement in an effort to reduce the risk of a hazardous level of self-heating~~ **pavement.(24).**

6.11.5 For Class II fills, use of drainage features located at the bottom of the fill that could provide free access to air should be avoided. This includes, but is not limited to, open graded drainage layers daylighting on the side of the fill. Under some conditions, it may be possible to use a well-graded granular soil as a drainage layer. The thickness of the drainage layer at the point where it daylighting on the side of the fill should be minimized. For TDA fills placed against walls, it is recommended that the drainage holes in the wall be covered with well-graded granular soil. The granular soil should be separated from the TDA with geotextile.

6.11.3 Embankments constructed in accordance with the guidelines have shown no evidence of ~~self-heating~~ self-heating **(2425).**

6.12 Type A TDA is a suitable alternative substitute for rock aggregate in on-site septic systems in ~~regards~~ regard to wastewater treatment and media durability **(2526).**

7. Material Specifications

7.1 The material specifications for TDA that are presented below take into consideration the need to limit internal heating of TDA fills as discussed in 6.11, producing a material that can be placed and compacted with conventional construction equipment, and limiting exposed steel belts to allow for rubber-to-rubber contacts between the pieces when placed in a fill. Moreover, TDA meeting the specifications can be produced with reasonably well-maintained processing equipment that has been properly selected for the size product being produced. Specifications are provided for two size ranges. The first is termed Type A and is suitable for many drainage, vibration damping, and insulation applications. The second is larger and is termed Type B. It is suitable for use as lightweight embankment fill, wall backfill, and some landfill drainage and gas collection applications.

7.1.1 The TDA shall be made from scrap tires which shall be shredded into the sizes specified in ~~7.1.27.1.3~~ 7.1.37.1.4 for Type A TDA or ~~7.1.37.1.4~~ for Type B TDA. They shall be produced by a shearing process. TDA produced by a hammer mill will not be allowed. The TDA shall be free of all contaminants including but not limited to oil, grease, gasoline, and diesel fuel that could leach into the groundwater or create a fire hazard. In no case shall the TDA contain the remains of tires that have been subjected to a fire, because the heat of a fire may liberate liquid petroleum products from the tire that could create a fire hazard when the TDA are placed in a fill. The TDA shall be free from organic matter such as fragments of wood, wood chips, and other fibrous organic matter. ~~The TDA shall have less than 1 % (by weight) of metal fragments that are not at least partially encased in rubber. Metal fragments that are partially encased in rubber shall protrude no more than 25 mm from the cut edge of the TDA on 75 % of the pieces (by weight) and no more than 50 mm on 90 % of the pieces (by weight). The gradation shall be measured in accordance with Test Method topsoil, etc. C136, except that the minimum sample size shall be 6 to 12 kg for Type A TDA and 16 to 23 kg for Type B TDA.~~

7.1.2 The TDA shall have less than 1 % (by weight) of metal fragments that are not at least partially encased in rubber. Metal fragments that are partially encased in rubber shall protrude no more than 25 mm from the cut edge of the TDA on 75 % of the pieces (by weight) and no more than 50 mm on 90 % of the pieces (by weight). The gradation shall be measured in accordance with Test Method C136/C136M, except that the minimum sample size shall be 6 to 12 kg for Type A TDA and 16 to 23 kg for Type B TDA.

7.1.3 Type A TDA shall have a maximum dimension, measured in any direction, of ~~200~~ 250 mm. In addition, Type A TDA shall have 100 % passing the 100-mm square mesh sieve, a minimum of 95 % passing (by weight) the 75-mm square mesh sieve, a maximum of ~~50 %~~ 70 % passing (by weight) the 38-mm square mesh sieve, and a maximum of 5 % passing (by weight) the 4.75-mm ~~sieve~~ sieve, as summarized in Table 2.

7.1.4 Type B TDA shall have a ~~minimum~~ maximum of ~~90 %~~ 16 % (by weight) with a maximum dimension, measured in any direction, of 300 mm and 100 % with a maximum dimension, measured in any direction, of 450 mm. At least one side wall shall be removed from the tread of each tire. The side wall will be considered removed if the bead wire has been completely severed from the side wall. A minimum of 75 % (by weight) shall pass the 200-mm square mesh sieve, a maximum of ~~50 %~~ 85 % (by

TABLE 2 TDA Gradation Requirements (27)

Sieve Opening (mm)	Sieve Opening (in.)	Type A Spec. Requirements (% passing)	Type B Spec. Requirements (% passing)
450	18	1	1
300	12	100 %	100 %
200	8	100 %	75–100 %
100	4	100 %	...
75	3	95–100 %	0–85 %
38	1.5	0–70 %	0–25 %
4.75	0.187 (No. 4)	0–5 %	0–1 %
pan	pan	0 %	0 %
Free steel		1 % max	1 % max
Longest shred (in.)		10	18
% weight of shred >12 in. long		...	16 % max
Sidewall shreds (ea)		0	0
Shreds >2 in. wire exposed		10 % max	10 % max
Shreds >1 in. wire exposed		25 % max	25 % max

weight) shall pass the 75-mm square mesh sieve, a maximum of 25 % (by weight) shall pass the 38-mm square mesh sieve, and a maximum of 1 % (by weight) shall pass the 4.75-mm sieve-sieve, as summarized in [Table 2](#).

8. Leachate

8.1 The Toxicity Characteristics Leaching Procedure (TCLP) (USEPA Method 1311) is one test to determine if a waste is regulated as a hazardous waste due to leaching of toxic compounds that could pose a significant hazard to human health. The TCLP test represents the scenario of acid rain percolating through the waste and exiting as leachate. For all regulated metals and organics, the results for TDA are well below the TCLP regulatory limits ([26-28-2830](#)); therefore, TDA are not classified as a hazardous waste.

8.2 In addition to TCLP tests, laboratory leaching studies have been performed following several test protocols. Results show that metals are leached most readily at low pH and that organics are leached most readily at high pH ([2830, 2931](#)). Thus, it is preferable to use TDA in environments with a near neutral pH.

8.3 The potential of TDA to generate leachate has been examined in field studies for both above- and below-groundwater table applications. The results have been compared to primary drinking water standards, secondary (aesthetic) drinking water standards, and USEPA preliminary remediation goals (PRG) ([3032](#)). PRG are risk-based concentrations that the USEPA considers to be protective for lifetime exposure to humans ([3032](#)). Freshwater aquatic toxicity has also been evaluated. These results were summarized in a literature review and statistical analysis performed for the USEPA Resource Conservation Challenge ([3133](#)).

8.4 In above-groundwater table applications, the TDA is placed above the water table and is subjected to water from infiltration. Seven field studies have examined this category of applications ([32-34-3941](#)). A statistical comparison was performed ([3133](#)) using procedures for censored environmental data recommended by Helsel ([4042](#)).

8.4.1 The preponderance of evidence shows that TDA used above the water table does not cause the primary drinking water standards for metals to be exceeded. Moreover, a statistical comparison shows that TDA is unlikely to increase levels of metals with primary drinking water standards above naturally occurring background levels ([3133](#)).

8.4.2 For above-groundwater table applications, it is likely that TDA would increase the concentrations of iron and manganese, which have secondary drinking water standards. At the point where water emerges from a TDA fill, it is likely that the levels of iron and manganese will exceed secondary drinking water standards, and the PRG for tap water for manganese will also be exceeded. After an extended dry period, an initial pulse of iron and manganese mass may occur ([4143](#)). When a TDA septic tank leach field serviced with typical domestic wastewater sewage was compared with a leach field comprised of rock aggregate media, iron, manganese, and zinc concentrations from the TDA effluent were statistically significantly higher compared to the rock media, which is likely a result of oxidation of metallic components in the TDA ([2526](#)). However, for two of three projects where samples were taken from wells adjacent to the TDA fills, the iron and manganese levels were about the same as background levels. The prevalence of manganese in groundwater is shown by the naturally occurring concentrations at three projects being above the secondary drinking water standard and PRG. For other chemicals with secondary drinking water standards, a statistical comparison shows that there is no evidence that TDA affects naturally occurring background levels ([3133](#)).

8.4.3 Volatile and semivolatile organics have been monitored on two projects where TDA was placed above the water table (33-35-3537). Substances are generally below detection limits. Moreover, for those substances with drinking water standards, the levels were below the standards. The concentrations were also below the applicable PRG (3133). A few substances were occasionally found above the test method detection limit; however, the highest concentrations were found in a control section located uphill from the TDA (3335), suggesting a source associated with active roadways. There are also laboratory studies showing that TDA has the ability to absorb some organic compounds (4244).

8.4.4 Aquatic toxicity tests were performed on samples taken from one above-groundwater table project. The results showed that water collected directly from TDA fills had no effect on survival, growth, and reproduction of two standard test species (fathead minnows and a small crustacean (*Ceriodaphnia dubia*)) (3133, 3436).

8.4.5 In summary, TDA placed above the water table would be expected to have a negligible off-site effect on water quality (3133).

8.5 TDA placed below the water table has been studied at three different sites (4345). A statistical comparison was performed (3133) using procedures for censored environmental data recommended by Helsel (4042).

8.5.1 A statistical analysis of the data at these sites showed that use of TDA did not cause primary drinking water standards for metals to be exceeded. Moreover, the data shows that TDA was unlikely to increase levels of metals with primary drinking water standards above naturally occurring background levels (3133).

8.5.2 For chemicals with secondary drinking water standards, it is likely that TDA below the groundwater table would increase the concentrations of iron, manganese, and zinc. For water that is collected directly from TDA fill below the groundwater table, it is likely that the concentrations of manganese and iron will exceed their secondary drinking water standards and PRG for tap water. The secondary drinking water standards and PRG for zinc were not exceeded even for water in direct contact with TDA. The rate at which metals leach from TDA is the highest when constantly submerged, but release rates decrease over time, where it significantly decreases after eight months and becomes constant by the end of 15 months at very low values; iron and manganese will likely be released from a submerged TDA fill at low, detectable rates for the lifetime of typical civil engineering applications (4143). The concentration of iron, manganese, and zinc decreases to near background levels by flowing only a short distance through soil (0.6 to 3.3 m). For other chemicals with secondary drinking water standards, a statistical comparison showed little likelihood that TDA placed below the water table alters naturally occurring background levels (3133).

8.5.3 Trace levels of a few volatile and semivolatile organics were found from water taken directly from TDA-filled trenches. The concentration of benzene, chloroethane, cis-1,2-dichloroethene, and aniline for water in direct contact with TDA are above their respective PRG for tap water. However, chloroethane, cis-1,2-dichloroethene, and aniline concentrations were below the PRG for all samples taken from wells 0.6 and 3.3 m downgradient. Moreover, the concentrations were below the detection limits for virtually all samples, indicating that these substances have limited downgradient mobility (2830).

8.5.4 The data on benzene deserves additional discussion. The primary drinking water standard for benzene is 5 µg/L and its PRG is 0.35 µg/L. For six sample dates, the detection limit reported by the laboratory was 0.5 µg/L, slightly above the PRG. For the remaining four sample dates the detection limit was 5 µg/L. Focusing on the data from samples with a detection limit of 0.5 µg/L, the benzene concentration was below the detection limit in downgradient wells for all but one well, on a single date, when the concentration was 1 µg/L. This data shows that benzene also has limited downgradient mobility (2830). In a different study where TDA was submerged in water for 15 months, the highest benzene concentration of 0.97 µg/L was observed at the beginning of the experiment, but dropped below detection limit of 0.3 µg/L by Week 34 (4143). This study indicated that the specific loss rates for benzene are highest at the beginning, and decline rapidly over the first 18 weeks (4143).

8.5.5 Aquatic toxicity tests were performed on samples taken on two dates. The results showed that water collected directly from TDA-filled trenches had no effect on survival and growth of fathead minnows. While there were some toxic effects of TDA placed below the groundwater table on *Ceriodaphnia dubia*, a small amount of dilution (up to threefold) as the groundwater flowed downgradient or when it entered a surface body of water would remove the toxic effects (3133, 3436).

8.5.6 In summary, TDA placed below the water table would be expected to have a negligible off-site effect on water quality (3133).

9. Keywords

9.1 construction practices; landfills; leachate; lightweight fill; rail lines; retaining walls; roads; scrap tires; TDA; tire chips; tire-derived aggregate; tire shreds; vibration damping

APPENDIX
(Nonmandatory Information)
X1. TYPICAL MATERIAL PROPERTIES

X1.1 This appendix contains typical properties of TDA to aid in the selection of values for preliminary designs and to provide a basis for comparison for test results.

X1.2 Values of specific gravity and water absorption capacity reported in the literature are summarized in [Table X1.1](#). The unit weight of TDA changes with placement and compaction conditions and the application of overburden stress, as summarized in [Table X1.2](#) (8). [Table X1.3](#) summarizes the compacted and uncompacted dry density of TDA. Compaction results for mixtures of TDA and soil also are available (4-6, [4446](#)). The results from one study are summarized in [Fig. X1.1](#).

X1.3 Typical compressibility results are summarized in [Table X1.4](#). The compressive properties between the different types of TDA are equivalent after initial compaction or compression ([2526](#)). Increased compressive loading results in a reduction in hydraulic conductivity.

X1.4 A measure of compressibility applicable to vehicle loads is resilient modulus. Results determined by Ahmed (5) using AASHTO T 274-82 for mixtures of TDA and soil are summarized in [Table X1.5](#). The parameter A, and therefore M_R , decreases as the percent TDA by dry weight of the mix increases. Results determined by Edil and Bosscher (4, [4951](#)) for mixtures of TDA and sand are summarized in [Fig. X1.2](#). Shao et al. ([5153](#)) performed resilient modulus tests on crumb rubber (7-mm (7 mm maximum size) and rubber buffings (1-mm (1 mm maximum size)). The resilient modulus values ranged from 700 to 1700 kPa.

X1.5 Typical values of coefficient of lateral earth pressure at rest and Poisson's ratio, measured as part of vertical compression tests, are presented in [Table X1.6](#).

X1.6 The shear strength of TDA has been measured using triaxial shear ([5](#), [4648](#), [5153](#)), simple shear ([13](#)), interface direct shear ([13](#)), and using direct shear ([12](#), [13](#), [4446](#), [4749](#), [5254](#)). [Tables X1.7-X1.9](#) [X1.12](#) summarize the Type B TDA shear test results of: simple shear testing of Type B TDA; internal interface direct shear testing of Type B TDA (DS); TDA and concrete interface direct shear testing of Type B TDA (DSI); TDA and sand interface direct shear testing of Type B TDA (DSIS); TDA and aggregate interface direct shear testing of Type B TDA (DSIA); and TDA and clay interface direct shear testing of Type B TDA (DSIC).

TABLE X1.1 Summary of Specific Gravity and Water Absorption Capacity

TDA Type	Specific Gravity			Water Absorption Capacity (%)	Reference
	Bulk	Saturate Surface Dry	Apparent		
Steel belted	1.06	1.01	1.10	4	(45)
Steel belted Mixture	1.06	1.01	1.10	4	(47)
Mixture	1.06	1.16	1.18	9.5	(46)
Mixture (Pine State)	---	---	1.18	9.5	(48)
Mixture (Pine State)	----	----	1.24	2	(44)
Mixture (Palmer)	----	----	1.24	2	(46)
Mixture (Palmer)	----	----	1.27	2	(44)
Mixture (Sawyer)	----	----	1.27	2	(46)
Mixture (Sawyer)	----	----	1.23	4.3	(44)
Mixture	1.01	1.05	1.23	4.3	(46)
Mixture	1.01	1.05	1.05	4	(45)
Mixture	1.01	1.05	1.05	4	(47)
Mixture (12.7 mm to 50.8 mm)	----	0.88 to 1.13	----	----	(5)