



Designation: ~~D7351~~—19 D7351/D7351M – 21

## Standard Test Method for Determination of Sediment Retention Device (SRD) Effectiveness in Sheet Flow Applications<sup>1</sup>

This standard is issued under the fixed designation ~~D7351~~; D7351/D7351M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon ( $\epsilon$ ) indicates an editorial change since the last revision or reapproval.

### 1. Scope—Scope\*

1.1 This test method establishes the guidelines, requirements and procedures for evaluating the ability of Sediment Retention Devices (SRDs) to retain sediment when exposed to sediment-laden water “sheet” flows.

1.2 This test method is applicable to the use of an SRD as a vertical permeable interceptor designed to remove suspended soil from overland, nonconcentrated water flow. The function of an SRD is to trap and allow settlement of soil particles from sediment laden water. The purpose is to reduce the transport of eroded soil from a disturbed site by water runoff.

1.3 The test method presented herein is intended to indicate representative performance and is not necessarily adequate for all purposes in view of the wide variety of possible sediments and performance objectives.

1.4 Units—The values stated in either SI units or inch-pound units [given in brackets] are to be regarded separately as standard. Only SI units are used in equations and appendixes. The values stated in each system may not be exact equivalents; therefore, each system shall be used independently of the other. Combining values from the two systems may result in nonconformance with the standard. Reporting of test results in units other than SI shall not be regarded as nonconformance with this standard.

1.5 All observed and calculated values shall conform to the guidelines for significant digits and rounding established in Practice [D6026](#).

1.5.1 The procedures used to specify how data are collected/recorded and calculated in this standard are regarded as the industry standard. In addition, they are representative of the significant digits that should generally be retained. The procedures used do not consider material variation, purpose for obtaining the data, special purpose studies, or any consideration for the user’s objectives; and it is common practice to increase or reduce significant digits of reported data to commensurate with these considerations. It is beyond the scope of this standard to consider significant digits used in analysis methods for engineering design.

1.6 *This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety, health, and environmental practices and determine the applicability of regulatory limitations prior to use.*

1.7 *This international standard was developed in accordance with internationally recognized principles on standardization established in the Decision on Principles for the Development of International Standards, Guides and Recommendations issued by the World Trade Organization Technical Barriers to Trade (TBT) Committee.*

<sup>1</sup> This test method is under the jurisdiction of ASTM Committee [D18](#) on Soil and Rock and is the direct responsibility of Subcommittee [D18.25](#) on Erosion and Sediment Control Technology.

Current edition approved ~~May 1, 2019~~ Aug. 15, 2021. Published ~~May 2019~~ August 2021. Originally approved in 2007. Last previous edition approved in ~~2013~~ 2019 as [D7351 – 13:19](#). DOI: ~~10.1520/D7351-19~~ 10.1520/D7351\_D7351M-21.

\*A Summary of Changes section appears at the end of this standard

## 2. Referenced Documents

### 2.1 ASTM Standards:<sup>2</sup>

- [D653 Terminology Relating to Soil, Rock, and Contained Fluids](#)
- [D698 Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort \(12,400 ft-lbf/ft<sup>3</sup> \(600 kN-m/m<sup>3</sup>\)\)](#)
- [D3740 Practice for Minimum Requirements for Agencies Engaged in Testing and/or Inspection of Soil and Rock as Used in Engineering Design and Construction](#)
- [D4318 Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils](#)
- [D4491/D4491M Test Methods for Water Permeability of Geotextiles by Permittivity](#)
- [D4632/D4632M Test Method for Grab Breaking Load and Elongation of Geotextiles](#)
- [D4751 Test Methods for Determining Apparent Opening Size of a Geotextile](#)
- [D4753 Guide for Evaluating, Selecting, and Specifying Balances and Standard Masses for Use in Soil, Rock, and Construction Materials Testing](#)
- [D5141 Test Method for Determining Filtering Efficiency and Flow Rate of the Filtration Component of a Sediment Retention Device](#)
- [D5261 Test Method for Measuring Mass per Unit Area of Geotextiles](#)
- [D6026 Practice for Using Significant Digits and Data Records in Geotechnical Data](#)
- [D6913/D6913M Test Methods for Particle-Size Distribution \(Gradation\) of Soils Using Sieve Analysis](#)
- [D7928 Test Method for Particle-Size Distribution \(Gradation\) of Fine-Grained Soils Using the Sedimentation \(Hydrometer\) Analysis](#)

## 3. Terminology

3.1 For definitions of terms used in this test method, see Terminology [D653](#). *Definitions:*

3.1.1 For definitions of common technical terms used in this standard, refer to Terminology [D653](#).

## 4. Summary of Test Method

4.1 Sediment-laden water is allowed to “sheet flow” up to and seep mixed in a tank and then discharged for 30 minutes as “sheet flow” down a slope. At the toe-of-slope the sheet flow reaches an installed sediment retention device (SRD). The flow then seeps through, over, or under, or a combination thereof, an installed sediment retention device (SRD), the SRD. At a minimum, the amount (via water and soil weight) of sediment-laden flow is measured both upstream and downstream of the SRD.

<https://standards.iteh.ai/catalog/standards/sist/1266fa8b-6e9a-4a1c-8547-6133e14a8136/astm-d7351-d7351m-21>  
4.2 The measurement of sediment that passes through, over, or under, or a combination thereof, the SRD compared to the amount in the upstream flow is used to quantify the effectiveness of the SRD in retaining sediments.<sup>3,4</sup>

## 5. Significance and Use

5.1 This test method quantifies the ~~ability~~ effectiveness of a sediment retention device (SRD) by measuring the ability of the SRD to retain eroded sediments caused by sheet flowing water under full-scale conditions. This test method may also assist in identifying physical attributes of SRDs that contribute to their erosion control performance.

5.2 The effectiveness of SRDs is installation dependent. Thus, replicating field installation techniques is an important aspect of this test method. This test method is full-scale and therefore, appropriate as an indication of product performance, for general comparison of product capabilities, and for assessment of product installation techniques.

NOTE 1—Test Method [D5141](#) is an alternate test method for evaluating sediment retention device effectiveness, if it is not necessary to simulate field installation conditions.

NOTE 2—The quality of the result produced by this standard is dependent on the competence of the personnel performing it, and the suitability of the equipment and facilities used. Agencies that meet the criteria of Practice [D3740](#) are generally considered capable of competent and objective

<sup>2</sup> For referenced ASTM standards, visit the ASTM website, [www.astm.org](http://www.astm.org), or contact ASTM Customer Service at [service@astm.org](mailto:service@astm.org). For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

<sup>3</sup> Sprague, C.J., “Testing the Effectiveness of Sediment Retention Devices,” *StormCon '04*, Palm Desert, CA, (digital proceedings), 2004.

<sup>4</sup> Sprague, C.J., and Carpenter, T., “A New Procedure for Testing the Effectiveness of Sediment Retention Devices,” *Conference XXXV*, International Erosion Control Assoc., Philadelphia, 2004, pp. 265–275.

testing/sampling/inspection/etc. Users of this standard are cautioned that compliance with Practice **D3740** does not in itself assure reliable results. Reliable results depend on many factors: Practice **D3740** provides a means of evaluating some of those factors.

**iTeh Standards**  
**(<https://standards.iteh.ai>)**  
**Document Preview**

[ASTM D7351/D7351M-21](#)

<https://standards.iteh.ai/catalog/standards/sist/1266fa8b-6e9a-4a1c-8547-6133e14a8136/astm-d7351-d7351m-21>

6. Apparatus

6.1 *Equipment required (see Fig. 1 and Fig. 2).*—The following equipment is required to perform testing in accordance with this standard (see Fig. 1 and Fig. 2)

6.1.1 *Combination Mixing Tank and Scale*—A tank with an internal paddle mixer device mounted on scales capable of holding/weighing 4500 kg [9920.8 lb] of sediment-laden water.

6.1.2 *A Clean Water Source and Pumping Equipment*—A source of water and associated pumping equipment sufficient to repeatedly fill the mixing tank in a timely manner.

6.1.3 *A Consistent Soil Stockpile*—A stockpile of soil in sufficient quantity to both create sediment-laden water and to create/replace subgrade in the installation zone. The general soil type to be used for testing shall be loam with target grain sizes and plasticity index as shown in Table 1, unless otherwise specified. Grain sizes (that is, particle size distribution) shall be determined in accordance with Test Methods D6913/D6913M and D7928, and plasticity index shall be determined in accordance with Test Methods D4318.

6.1.4 *A Loader for Moving the Soil to the Mixer*—A front-end loader of sufficient reach and capacity to dump a prescribed amount of soil into the mixing tank.

6.1.5 *A Variable Discharge Apparatus from the Mixer*—A variable discharge apparatus from the mixer — A valve-controlled discharge hose that allows for controlled, uniform discharge from the mixing tank.

6.1.6 *Soil and Water Sampling Equipment*—Sampling jars (at least 12 per test) for taking “grab” samples periodically during the test.

6.1.7 *Excavating/Compacting Machinery for Cleaning and Preparing the Test Area*—Earthmoving and compacting equipment is needed to prepare/replace the soil in the installation zone.

6.1.8 *A Scaled Collection System Adequate to Handle the Released Runoff*—A tank mounted on scales of sufficient volume to collect all runoff passing the SRD.

6.2 Retention Area:

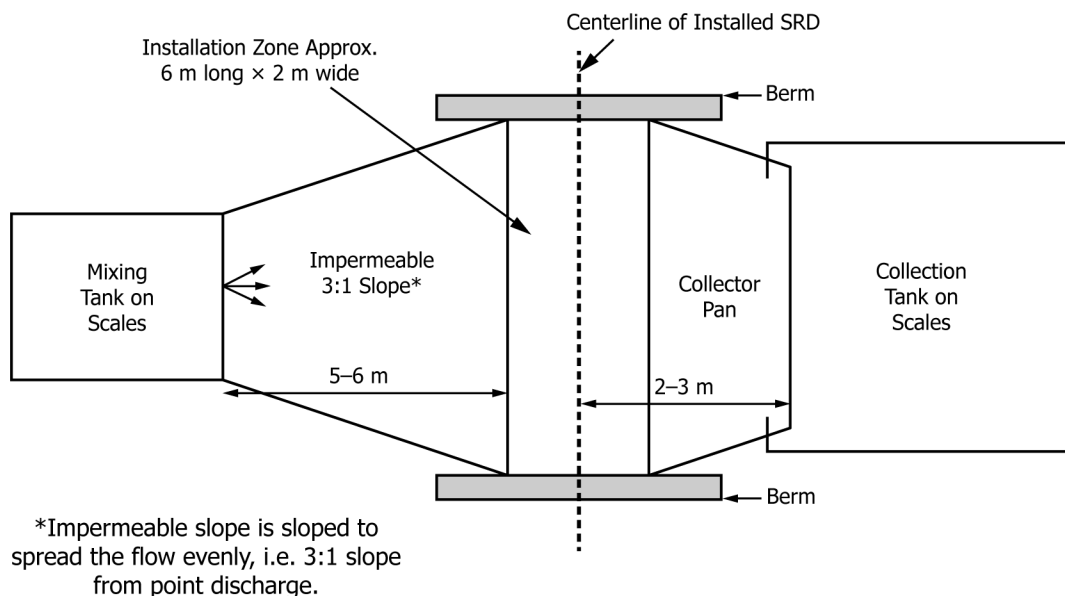


FIG. 1 Profile Schematic Schematic (Plan) Diagram

Diagram of Proposed Testing Equipment  
(not to scale)

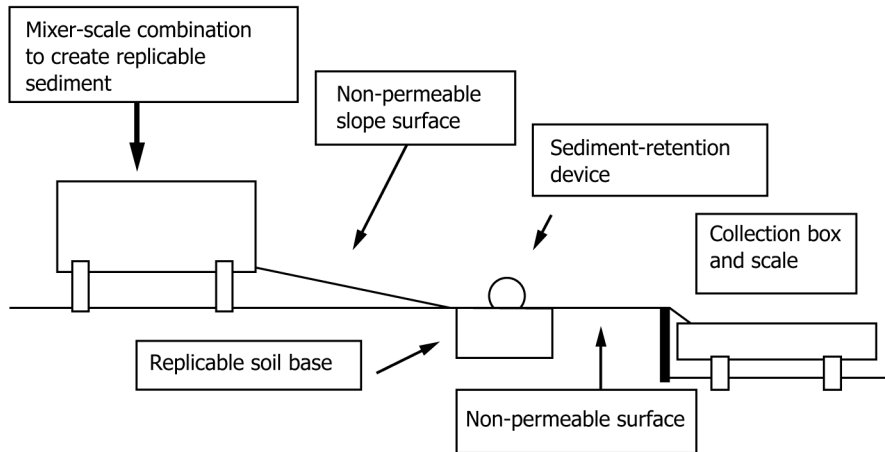


FIG. 2 Schematic (Plan)(Profile) Diagram

TABLE 1 Target Grain Sizes and Plasticity Indices

| Particle Size    | Sand                  | Loam                    | Clay                      |
|------------------|-----------------------|-------------------------|---------------------------|
| $D_{100}$ (mm)   | $25 > D_{100} > 3.0$  | $10 > D_{100} > 0.3$    | $3.0 > D_{100} > 0.02$    |
| $D_{85}$ (mm)    | $4.0 > D_{85} > 0.8$  | $0.8 > D_{85} > 0.08$   | $0.08 > D_{85} > 0.003$   |
| $D_{65}$ (mm)    | $4.0 > D_{65} > 0.8$  | $0.8 > D_{65} > 0.08$   | $0.08 > D_{65} > 0.003$   |
| $D_{50}$ (mm)    | $0.9 > D_{50} > 0.2$  | $0.15 > D_{50} > 0.015$ | $0.015 > D_{50} > 0.0008$ |
| $D_{15}$ (mm)    | $0.3 > D_{15} > 0.01$ | $0.03 > D_{15} > 0.001$ | $D_{15} > 0.002$          |
| $D_{15}$ (mm)    | $0.3 > D_{15} > 0.01$ | $0.03 > D_{15} > 0.001$ | $D_{15} < 0.002$          |
| Plasticity Index | N/A (nonplastic)      | $2 < PI < 8$            | $10 < PI$                 |

(<https://standards.iteh.ai>)  
Document Preview

6.2.1 A non-permeable, smooth, 3:1 slope surface (at least 55.0 m [16.4 ft] long) immediately below the mixer discharge shall be provided to spread the discharge to the width of the retention zone (length of the SRD installation) and to provide a retention zone above the installation zone.

6.2.2 An installation zone approximately 2 m [6.6 ft] wide by the intended length of the SRD installation (typically 20 ft) 6.1 m [20 ft] comprised of prepared soil subgrade to allow full-scale installation of the SRD to be tested. Berms or walls of sufficient height to contain ponded water are constructed at the ends of the installation zone as shown in Fig. 1.

6.2.3 The center of the installed SRD should be placed in the center of the installation zone each time to replicate height of water as it relates to volume retained.

6.2.4 The prepared soil subgrade will be compromised each test, so it will have to be reconstructed after each test.

6.2.5 The area below the installation zone should be non-permeable to facilitate efficient transmission of runoff passing the SRD to the collection tank.

6.3 The Collection Area:

6.3.1 The collection tank shall be at a lower grade than the installation area so that runoff passing the SRD will efficiently flow via gravity into the tank. A retaining wall between the installation zone and the collection tank is recommended. The area between the retaining wall and the installed SRD should be impermeable, and so facilitate collection of sediments deposited after passing the SRD but before entering the collection tank.

6.4 Balance—Balances shall conform to the requirements of Guide D4753.

6.4.1 Use a calibrated balance or scale capable of determining mass to an accuracy of 50 g [0.1 lb] when measuring bulk sediment mass.

6.4.2 Use a calibrated balance or scale capable of determining mass to an accuracy of 0.01 g [0.00002 lb] when measuring bottle sample masses.

6.5 *Steel Rule*—Use a steel rule capable of measuring to an accuracy of 1mm [0.003 ft].

**7. SRD Properties and Installation**

7.1 A single representative sample of the SRD to be tested shall be used. The sample shall be a minimum of 6.1 m [20.0 ft] in length.

7.2 Obtain a sufficient size sample of the product to be tested and submit the sample for the index tests shown in [Table 2](#) or [Table 3](#), unless other tests are requested by the client.

7.3 The SRD sample shall be installed in accordance with the manufacture’s recommendations or, lacking recommendations, in accordance with generally accepted construction procedures, including orientation perpendicular to flow with appropriate trenching and/or staking.

**8. Procedure**

8.1 *SRD Installation:*

8.1.1 Prepare the installation zone using the same soil to be used as sediment, unless otherwise agreed with the client. The soil shall be placed to a depth in excess of the depth of installation and compacted to  $90 \pm 3 \%$  of Standard Proctor density, at a soil moisture within  $\pm 3 \%$  of optimum moisture content in accordance with Test Method [D698](#), unless otherwise requested by the client. The installation zone should be wide enough to accommodate the desired length of SRD. Unless otherwise agreed with the client, the SRD length exposed to flow between end abutments shall be sufficient to completely contain the test flow, but no more than the 7 m.

8.2 *Mixing, Releasing, and Collecting Sediment-Laden Runoff:*

8.2.1 Procure soil as described in [6.1.3](#) in adequate quantities for the testing process, determine its characteristics for future replication needs, and cover to prevent contamination and rainfall degradation.

8.2.2 Create sediment-laden runoff by combining water and soil in the mixing tank and maintain agitation during the test. The quantities of water and soil to create the sediment-laden runoff shall be 2140 kg [4700 lb] of water and 140 kg [300 lb] of soil (see [Note 3](#) ~~Note 3~~), except in those cases where the client directs the use of project-specific quantities calculated in accordance with [Annex A1](#). The amount of water and sediment simulates sheet flow from a slope measuring 6.1 m [20.0 ft] wide by 30 m [98.4 ft] long. A proportionate amount of water and soil (15.67:1) should be used for testing if exposing more or less than 66.1 m [20.0 ft] of SRD at the toe of the slope.

NOTE 3—When using a standard 10-y, 6-h storm event for the mid-Atlantic region of US it can be assumed that 100 mm [4 in.] rainfall will occur. It was also assumed that approximately 25 % of the storm would occur during the peak 30 minutes, and that 50 % of the rainfall would infiltrate into the ground. The runoff and sediment load is then calculated as follows:

The following calculations provided the runoff and sediment load used in the testing:

$$T = 89.6 (V \times Q_p)^{0.56} K L S C P$$

where:

$$V = (0.5 \times 0.1 \text{ m}) \times 180 \text{ m}^2 = 9 \text{ m}^3$$

**TABLE 2 Index Tests for Tested Product: Silt Fence—Type SRD**

| Silt Fence—Type SRD |                              |
|---------------------|------------------------------|
| Mass/Area           | <a href="#">D5261</a>        |
| Tensile Strength    | <a href="#">D4632/D4632M</a> |
| AOS                 | <a href="#">D4751</a>        |
| Permittivity        | <a href="#">D4491/D4491M</a> |



TABLE 3 Index Tests for Tested Product: Wattle/Sock—Type SRD

| Wattle/Sock—Type SRD <sup>A</sup> |                              |
|-----------------------------------|------------------------------|
| Mass/Length                       | calibrated ruler and balance |
| Circumference                     | calibrated ruler             |
| Nominal Height                    | calibrated ruler             |

<sup>A</sup>Testing after 24 hours at ambient conditions.

$$Q_p = (0.1 \text{ m}) \times (0.25)^* \times (0.5)^{**} \times (180 \text{ m}^2) = 2.25 \text{ m}^3 / 30 \text{ min} = 0.00125 \text{ m}^3/\text{s} \text{ (*} = 25 \% \text{ of storm during 30-min peak; **} = 50 \% \text{ infiltration)}$$

$$K, \text{ sandy-silt} = 0.041; \text{LS, 2-5 \%}/30 \text{ m} = 0.46; \text{C, P} = 1.0$$

$$T = 89.6 (9 \times 0.00125)^{0.56} (0.041) (0.46) (1.0) (1.0) = 0.137 \text{ Tonnes} = 137 \text{ kg of soil (assume most sediment is generated during the peak flow period)}$$

Test Quantities:

30-Minute Runoff:  $2.25 \text{ m}^3 \times 1000 \text{ kg/m}^3 = 2250 \text{ kg}$

Sediment Load: 136 kg

(This hypothetical combination of water and sediment produces a sediment concentration slightly below 6 %. It is recommended that quantities be rounded to achieve a 6 % sediment concentration. Thus, it is recommended that default testing be done with 2140 kg (4700 lb) of water and 140 kg (300 lb) of dry soil.)

8.2.3 Discharge volume evenly for 30 min. Measure the quantity of released runoff at no less than 5-min intervals by noting the reduction in mass in the mixing tank to the nearest 0.5 kg [1 lb]. Adjust the valve on the outlet hose to increase/decrease flow to stay as close as possible to the target (that is, 2280 kg [5026.5 lb] / 30 min = 76 kg [167.6 lb] / min). Maintaining a relatively steady and accurate flow rate is important, as the calculations will assume that this flow rate is constant. The retention area construction and mixing tank discharge should combine to cause the flow to spread out to impact the full length of the SRD

NOTE 4—Larger flow rates than defined in 8.2.2 may not result in sheet flow impacting the SRD. If this occurs this test method is not appropriate.

8.2.4 As runoff passing the SRD enters the collection tank, record the weight/mass of the collection tank, to the nearest 0.5 kg [1 lb], and take grab samples of runoff entering the tank, at 5 min intervals.

8.2.5 Cutoff time is 90 min, unless otherwise directed by the client. An earlier cutoff time is acceptable when there is low-volume ponding and minimal discharge.

### 8.3 Data Collection:

8.3.1 *Grab Samples*—Collect grab samples at 5 min intervals at the mouth of the discharge from the mixing tank and from a suitable downstream (between the installed SRD and the collection tank) location. Collect all samples in the same size container (250 mL [0.07 gal] bottles are recommended) and in the same manner. Pre-mark each container and do not overfill or overrun the sample bottle. Concentrations may be small, thus poor sampling techniques may significantly affect results. Multiple measurements cause a hectic pace, so pre-marking and immediately recording insures consistency and accuracy.

8.3.2 *Tank Measurements*—Sediment-laden water in both the mixing and collection tanks is primarily measured by weight/mass, to the nearest 0.5 kg [1 lb].

8.3.3 *Observations*—Make and record visual observations relevant to the testing, such as height of the SRD, the depth of ponding (maximum depth and depth at end of test), undermining, overtopping, catastrophic product/system failure, etc. and the associated times. It is important to document the progression of undermining flows and associated times. Photographic and video documentation is preferred.

8.3.4 *Time Records*—Record the time of each grab sample, each tank and ponding depth measurement, measurement (to the nearest 1 mm [0.003 ft]), and the time to reach zero ponding height, if reached within the time of the entire test.

8.3.5 *Sediments*—Collect and dry all sediments passing the SRD.

### 8.4 Lab Testing:

8.4.1 *Turbidity*—Grab samples shall be evaluated in a lab to determine turbidity and turbidity value recorded to the nearest 1 NTU.