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Designation: D 6723 – 01

Standard Test Method for Determining the Fracture Properties of Asphalt Binder in Direct Tension (DT)¹

This standard is issued under the fixed designation D 6723; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ϵ) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 This test method covers the determination of the failure strain and failure stress of asphalt binders by means of a direct tension test. It can be used with unaged material or with material aged using Test Method D 2872 (RTFOT), Practice D 6521 (PAV), or AASHTO T240 (RTFOT) and AASHTO PP1 (PAV). The test apparatus is designed for testing within the temperature range from +6 to -36° C.

1.2 This test method is limited to asphalt binders containing particulate material having dimensions less than 250 µm.

1.3 The values stated in SI units are to be regarded as the standard.

1.4 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

2. Referenced Documents

2.1 ASTM Standards:

- C 670 Practice for Preparing Precision and Bias Statements for Test Methods for Construction Materials²
- D 8 Terminology Relating to Materials for Roads and Pavements³
- D 140 Practice for Sampling Bituminous Materials³
- D 2872 Test Method for Effect of Heat and Air on a Moving Film of Asphalt (Rolling Thin-Film Oven Test)³
- D 6373 Specification for Performance Graded Asphalt Binder³

D 6521 Practice for Accelerated Aging of Asphalt Binder Using a Pressurized Aging Vessel (PAV)³

- E 1 Specification for ASTM Thermometers⁴
- E 4 Practices for Force Verification of Testing Machines⁵
- E 77 Test Method for the Inspection and Verification of Thermometers 4

- ² Annual Book of ASTM Standards, Vol 04.02.
- ³ Annual Book of ASTM Standards, Vol 04.03.
- ⁴ Annual Book of ASTM Standards, Vol 14.03.

E 83 Practice for Verification and Classification of Extensometers⁵

- E 220 Test Method for Calibration of Thermocouples by Comparison Techniques⁴
- 2.2 Deutche Industrie Norm (DIN) Standard:
- 43760 Standard for Calibrating Thermocouples⁶
- 2.3 AASHTO Standards:
- T 240⁷
- PPI (PAV)⁷

3. Terminology

3.1 *Definitions*—For definitions of general terms used in this standard, refer to Terminology D 8.

3.2 Definitions of Terms Specific to This Standard:

3.2.1 *brittle*, *adj*—type of failure in a direct tension test where the stress-strain curve is essentially linear up to the point of failure and the failure is by sudden rupture of the test specimen without appreciable reduction in cross-section of the specimen.

3.2.2 *brittle-ductile*, *adj*—type of failure in a direct tension test where the stress-strain curve is curvilinear and the failure is by sudden rupture of the test specimen. Limited reduction in cross-section of the specimen occurs before rupture.

3.2.3 *ductile*, *adj*—type of failure in a direct tension test where the specimen does not rupture but fails by flow at large strains.

3.2.4 effective gage length, n—for specimens used in this test, the effective gauge length, L_e , has been determined to be 33.8 mm. This is an effective gauge length that represents the portion of the specimen that contributes to the majority of the strain.

3.2.5 *failure*, *n*—point at which the tensile load reaches a maximum value as the test specimen is pulled at a constant rate of elongation.

3.2.6 *failure strain*, *n*—the tensile strain corresponding to the failure stress.

3.2.7 *failure stress*, *n*—the tensile stress on the test specimen when the load reaches a maximum value during the test method specified in this standard.

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¹ This test method is under the jurisdiction of ASTM Committee D04 on Road and Paving Materials and is the direct responsibility of Subcommittee D04.44 on Rheological Tests.

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⁵ Annual Book of ASTM Standards, Vol 03.01.

⁶ Deutsches Institut fur Normung e.V. (German Standards Institute), Beuth Verlag GmbH, Burggrafenstrasse 6, 10787, Berlin Germany.

⁷ Available from the American State Highway and Transportation Officials, 444 N. Capitol Street, N.W., Washington, DC 20001.

3.2.8 gage section, n—the central portion of the specimen where the cross-section does not change with length. For the geometry used in this test method, the gage section is 18 mm in length (see Fig. 1).

3.2.9 *tensile strain*, n—axial strain resulting from the application of a tensile load and calculated as the change in length of the effective gage length caused by the application of the tensile load divided by the original unloaded effective gage length.

3.2.10 *tensile stress*, *n*—axial stress resulting from the application of a tensile load and calculated as the tensile load divided by the original area of cross-section of the specimen.

4. Summary of Test Method

4.1 This test method describes the procedure used to measure the stress at failure and strain at failure in an asphalt binder test specimen pulled at a constant rate of elongation. Test specimens are prepared by pouring hot asphalt binder into a suitable mold. Two plastic end tabs are used to bond the asphalt binder during the test and to transfer the tensile load from the test machine to the asphalt binder.

4.2 This test method was developed for asphalt binders at temperatures where they exhibit brittle or brittle-ductile failure. A brittle or brittle-ductile failure will result in a fracture of the test specimen as opposed to a ductile failure in which the specimen simply stretches without fracturing. The test method is not applicable at temperatures where failure is by ductile flow.

4.3 A displacement transducer is used to measure the elongation of the test specimen as it is pulled in tension at a constant rate of 1.00 mm/min. The load developed during the test is monitored and the tensile strain and stress in the test specimen when the load reaches a maximum are reported as the failure strain and failure stress, respectively.

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5. Significance and Use

5.1 Stress at failure is used in a mechanistic pavement cracking model to compute critical cracking temperature. The procedure to compute critical cracking temperature is described in Practice D 6521. The critical cracking temperature is then used in specifying the low temperature grade of asphalt binder in accordance with Specification D 6373.

5.2 The test method is designed to measure the strength of the asphalt binder at the critical cracking temperature. The asphalt binder has limited ability to resist stress without cracking. In the asphalt binder Specification D 6373, failure stress is used to determine the critical cracking temperature.

5.3 For evaluating an asphalt binder for conformance to Specification D 6373, the elongation rate of the gage section is 1.0 mm/min and the test temperature is selected from Table number 1 of Specification D 6373 according to the grade of asphalt binder. Other rates of elongation and test temperatures may be used to test asphalt binders.

6. Apparatus

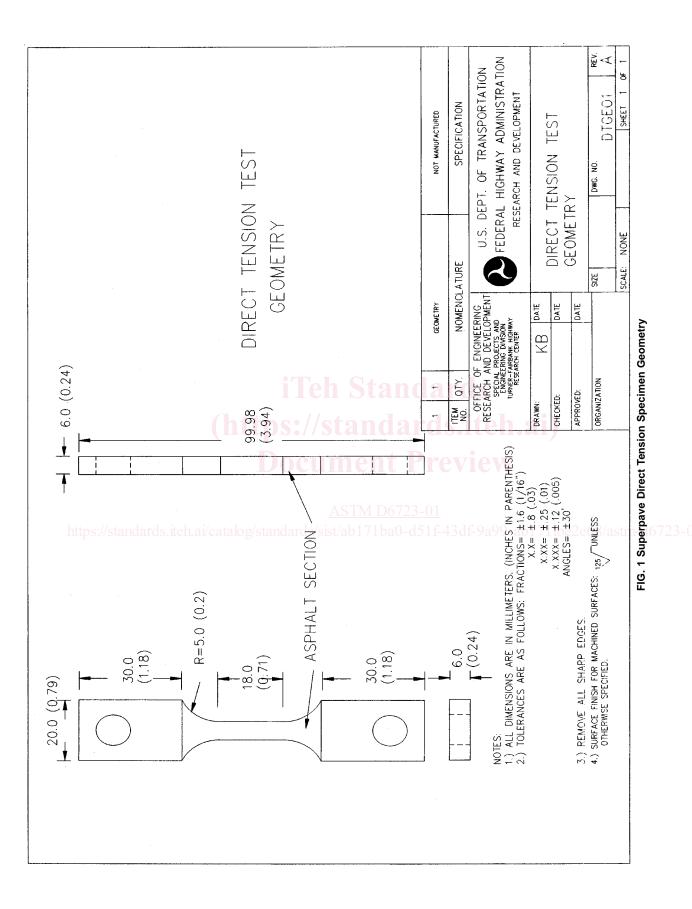
6.1 Direct Tension Test System—A direct tension test system consisting of (1) a closed feed-back loop displacement-controlled tensile loading machine, (2) a specimen gripping system, (3) either a fluid bath or an insulated chamber for

reliable, accurate and uniform temperature control during testing and conditioning of specimen, (4) real-time load measuring and recording devices, (5) real-time elongation measuring and recording devices, (6) a real-time temperature detection and recording device, and (7) real-time data acquisition and display devices. The system shall have an electromechanical or servo-hydraulic loading unit capable of applying and measuring tension and compression forces of at least 500 N and actuator travel of 20 mm. The system stiffness shall be at least 3 MN/m including the load cell and the loading pins. The unit shall have a transducer to measure and control grip separation and provide a feedback for strain control with a displacement resolution of 1.0 µm. The system shall be capable of closed loop elongation rate control accurate to at least 1 % of the commanded specimen elongation rate using feedback from a displacement transducer mounted between the loading pins or a non-contact extensometer measuring the elongation of the specimen.

6.1.1 Tensile Loading Machine Equipped With Temperature Control-A tensile loading machine with a controlleddisplacement loading frame capable of producing at least a 500.00 N load is required. The loading frame shall be table mounted. The gripping system (loading pins and platens) shall be completely submerged under the cooling fluid if a fluid based system is used. The gripping system shall be a minimum of 25 mm under the cooling fluid surface. Loading shall be accomplished by pulling directly in tension in the plane of the specimen. The distance between the load frame's loading points (between loading pins) shall accommodate specimens with total length (including the end tabs) of at least 100.0 mm, (see Fig. 1). If an air cooled system is used, the testing frame shall be equipped with two standards (columns) with sufficient clear space between the standards so that an insulated temperature control chamber can be placed between the standards.

6.1.2 Specimen Gripping System—The gripping system must produce a self-aligning uniaxial test load and accept the end tabs described in 6.1.2.1 and be designed so that test specimens can be easily mounted in the machine. The system shall include two grips. Each grip shall include a specially-shaped pin that is mounted rigidly to the loading platens of the testing machine. Fig. 2 shows a typical grip and loading pin assembly. One grip shall be fixed and remain stationary during the test while the other grip is displaced at the desired elongation rate.

6.1.2.1 Specimen End Tabs—End tabs made from Phenolic G-10 material having the dimensions specified in Figs. 3-5 shall be bonded to both ends of the test specimen to transfer the tensile load to the asphalt binder. The end tabs shall be machined from standard G-10 Phenolic sheet. Each end tab shall contain a precisely machined hole lined with a 304 stainless steel ring. The diameter of the lined hole shall be 10.00 \pm 0.05 mm. Gripping of the specimen is accomplished through the bond (adhesion) between the asphalt binder test specimen and the end tab. Each end tab shall be mounted on a specially shaped pin that is part of the gripping system. The specimen shall be mounted on the grips by positioning the end tabs in the test machine such that the end tabs fit onto the pins and are indexed against the face of the grips. Matching the



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FEDERAL HIGHWAY ADMINISTRATION Þ¢ ы Ч U.S. DEPT. OF TRANSPORTATION RESEARCH AND DEVELOPMENT SHEET 1 SPECIFICATION DTLPG1 316 STAINLESS STEEL જે DWG. NO. <u>Z</u> FONDING ├---------SCALE: NONE 2 NOMENCLATURE PRESS SIZE OFFICE OF ENGINEERING RESEARCH AND DEVELOPMENT SPECIAL PROJECTS AND ENGINEERING DIVISION TUNKERING DIVISION TUNKERIC DIVISION DATE DATE DATE GRIP $\stackrel{\times}{\simeq}$ -FITTED QTY. ORGANIZATION 3 APPROVED: CHECKED: DRAWN: NON ----PRESS- TOLERANCES ARE IN MILLIMETERS. (INCHES IN PARENTHESIS)
TOLERANCES ARE AS FOLLOWS: FRACTIONS= ±1.6 (1/16")
X.X = ±.8 (.03)
X.X = ±.12 (.05)
X.X = ±1.2 (.005) (0.59)ND 15.0 0 3.) REMOVE ALL SHARP EDGES. 4.) SURFACE FINISH FOR MACHINELD SURFACES: 125/ UNLESS OTHERWISE SPECIFIED. TYPICAL GRIP SHOWING THE TWO GRIPS ARE NEEDED. 15.0 (0.59) -10.0 (0.39) 12.0 (0.47) 15.0(0.59) Π (0.47 <u>+</u> 0.079)¹ 12.0 ± 2.0 NOTES:

FIG. 2 Loading Pin and Grip Assembly for the Superpave Direct Tension Test

FIG. 3 Superpave End Insert for Direct Tension Test (See Fig. 4 for the Metal Ring Dimensions to be Press Fitted into this End Insert) FEDERAL HIGHWAY ADMINISTRATION ž \triangleleft U.S. DEPT. OF TRANSPORTATION Ь RING RESEARCH AND DEVELOPMENT DWG. NO. DTINS41 SPECIFICATION SHEET END TAB FOR METAL SCALE: NONE NOMENCLATURE SIZE Ring and Assembly Drawings. **1** 6.0 OFFICE OF ENSINEERING RESEARCH AND DEVELOPMENT ERGNEERING DIVISION UNRELARENG DIVISION UNRELARENG DIVISION DATE DATE DATE щ ITEM QTY. RGANIZATION 1 APPROVED: CHECKED: DRAWN: G-10 Phenolic 12.675 12.625 NOTES: 1) ALL DIMENSIONS ARE IN MILLIMETERS. (INCHES IN PARENTHESIS) 2.) TOLERANCES ARE AS FOLLOWS: FRACTIONS= ±1.6 (0.3) XXX= ±1.25 (0.0) XXXX= ±1.2 (0.05) ANGLES= ±30 Q 0.01 ----10.0 3.) REMOVE ALL SHARP EDGES. 4.) SURFACE FUISH FOR MACHINED SURFACES: 125/UNLESS 07HERWISE SPECIFIED. Also See Metal . . 30.0 -MATERIAL NZ/TYP. NOTES: 2. 20.0

FEDERAL HIGHWAY ADMINISTRATION REV \triangleleft 5 U.S. DEPT. OF TRANSPORTATION U A D * RESEARCH AND DEVELOPMENT SPECIFICATION DTINS42 SHEET RING, END DWG. NO. SCALE: NONE NOMENCLATURE SIZE OFFICE OF ENGINEERING RESEARCH AND DEVELOPMENT SPECIA PROJECTS AND ENGINEERING DIVISION TURNEERING DIVISION TURNEERING CONTER FIG. 4 Superpave End Insert Metal Ring for Direct Tension Test DATE DATE DATE Ш¥ Ø 10,05 ITEM QTY. NO. 9,95 ORGANIZATION APPROVED: CHECKED: DRAWN: 40 304 2.) TOLERANCES ARE AS FOLLOWS: FRACTIONS= ±1.6 (1/16") 2.) TOLERANCES ARE AS FOLLOWS: FRACTIONS= ±1.6 (1/16") 2.) X.X.= ±.25 (.01) X.XX.= ±.12 (.005) ANGLES= ±30' SCHEDULE Ω Z 316 OR REMOVE ALL SHARP EDGES.
SURFACE FINISH FOR MACHINED SURFACES: 125 UNLESS 0THERWISE SPECIFIED. MAY BE MADE FROM 1/4" ST. ST. TYPE SEAMLESS PIPE MATERIAL: N NOTES: NOTES: Ø 12,700 12,725