



Designation: E 2126 – 02

Standard Test Methods for Cyclic (Reversed) Load Test for Shear Resistance of Framed Walls for Buildings¹

This standard is issued under the fixed designation E 2126; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 These test methods are designed to evaluate the shear stiffness, shear strength, and ductility of a wall assembly, including applicable shear connections and hold-down connectors, under quasi-static cyclic (reversed) load conditions.

1.2 These test methods are intended for wall assemblies constructed from wood or metal framing with solid sheathing or other bracing methods.

1.3 *This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.*

2. Referenced Documents

2.1 ASTM Standards:

- D 2395 Test Methods for Specific Gravity of Wood and Wood-Based Materials²
- D 4442 Test Methods for Direct Moisture Content Measurement of Wood and Wood-Based Materials²
- D 4444 Test Methods for Use and Calibration of Hand-Held Moisture Meters²
- E 564 Practice for Static Load Test for Shear Resistance of Framed Walls for Buildings³
- E 575 Practice for Reporting Data from Structural Tests of Building Constructions, Connections, and Assemblies³

3. Terminology

3.1 Definitions of Terms Specific to This Standard:

3.1.1 *ductility factor* (μ), n —the ratio of the ultimate displacement (Δ_u) and the yield displacement (Δ_{yield}).

3.1.2 *envelope curve* (see Fig. 1), n —the locus of extremities of the load-displacement hysteresis loops. Initial envelope curve contains the peak loads from the first cycle of each phase of the cyclic loading. Wall displacement in the positive

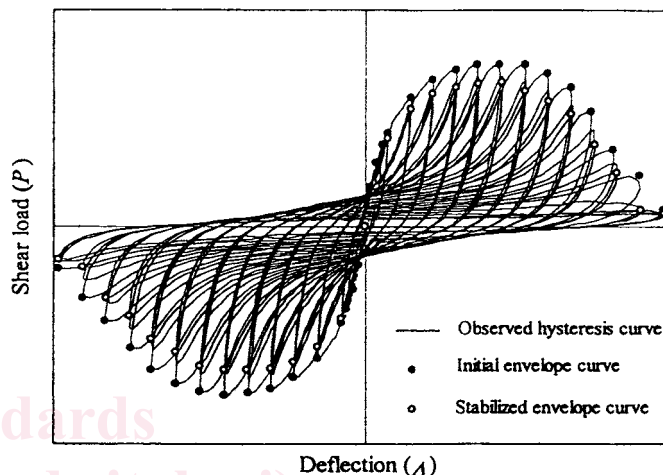


FIG. 1 Observed Hysteresis Curve and Envelope Curves

direction produces a positive envelope curve; the negative wall displacement produces a negative envelope curve.

3.1.3 *equivalent energy elastic-plastic (EEEP) curve* (see Fig. 2), n —an ideal elastic-plastic curve circumscribing an area equal to the area enclosed by the observed load-displacement curve or envelope curve between the origin, the ultimate displacement, and the displacement axis. The elastic portion of the EEEP curve contains the origin and has a slope equal to the elastic stiffness, k_e . The plastic portion is a horizontal line equal to P_{yield} determined by the following equation:

$$P_{yield} = \left(\Delta_u - \sqrt{\Delta_u^2 - \frac{2A}{k_e}} \right) k_e \quad (1)$$

where:

- P_{yield} = yield load (lbf or N);
- A = the area (lbf-in. or N-m) under the observed load-displacement curve or envelope curve from zero to ultimate displacement (Δ_u);
- k_e = elastic shear stiffness (lbf/in. or N/m) defined by the slope of the secant passing through the origin and a point on the observed load-displacement curve or envelope curve where the load equals $0.4 P_{peak}$.

3.1.4 *failure limit state*, n —the point in the load-displacement relationship corresponding to the last data point with the absolute load equal or greater than $0.8 P_{peak}$.

¹ These test methods are under the jurisdiction of ASTM Committee E06 on Performance of Buildings and are the direct responsibility of Subcommittee E06.11 on Horizontal and Vertical Structures/Structural Performance of Completed Structures.

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² Annual Book of ASTM Standards, Vol 04.10.

³ Annual Book of ASTM Standards, Vol 04.11.

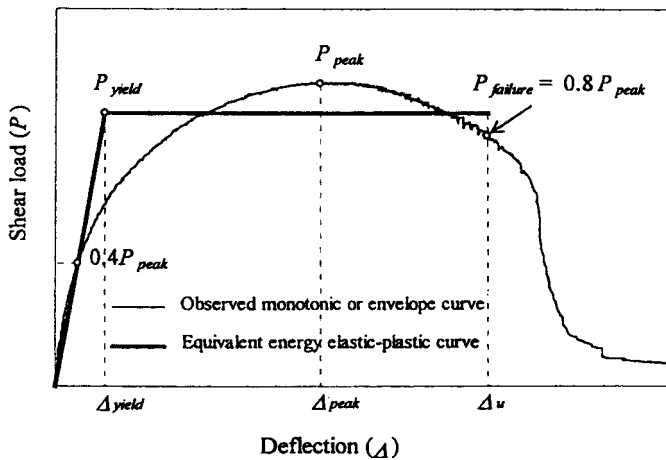


FIG. 2 Performance Parameters of Shear-Wall Assembly

3.1.5 *failure load*, $n=0.8 P_{peak}$.

3.1.6 *first major event (FME)*, n —the first significant limit state to occur (see *limit state*).

3.1.7 *limit state*, n —an event that marks the demarcation between two behavior states, at which time some structural behavior of the element or system is altered significantly.

3.1.8 *strength limit state*, n —the point in the force-displacement relationship corresponding to the maximum absolute displacement at Δ_{peak} for the maximum absolute load (P_{peak}) resisted by the assembly.

3.1.9 *ultimate displacement (Δ_u)*, n —the displacement corresponding to the failure limit state.

3.1.10 *yield limit state*, n —the point in the load-displacement relationship where the elastic shear stiffness of the assembly decreases 5 % or more. For assemblies with nonlinear ductile elastic response, the yield point ($\Delta_{yield}, P_{yield}$) is permitted to be determined using the equivalent energy elastic-plastic curve (see 3.1.3).

4. Summary of Test Method

4.1 The cyclic shear stiffness, shear strength and ductility of walls are determined by subjecting a wall assembly to full-reversal cyclic racking shear loads. This is accomplished by anchoring the bottom edge of the wall assembly to a rigid base and applying a force parallel to the top of the wall. The test assembly is allowed to displace in its own plane. As the wall assembly is racked to specified displacement increments, the racking (shear) force and displacements are continuously measured (see 8.6).

5. Significance and Use

5.1 These test methods are intended to measure the performance of framed walls subjected to earthquake loads. Since these loads are dynamic and cyclic, the loading process simulates the actions and their effects on the walls.

6. Wall Assembly

6.1 *General*—The typical wall assembly consists of a frame on which the elements comprising the wall, including the sheathing (or diagonal bracing members, if applicable) are placed. The elements shall be fastened to the frame in a manner

to conform with 6.2. Elements used to construct wall assemblies may be varied to permit anticipated failure of selected elements. Frameless wall assemblies are also permitted to be tested under the guidelines of these test methods.

6.2 *Connections*—The performance of framed walls is influenced by the type, spacing and edge distance of fasteners attaching sheathing to framing and spacing of the shear connections and hold-down connectors to the rigid base. All of these connections shall be consistent with the types used in actual building connections.

6.3 *Frame Requirements*—The frame of the wall assembly shall consist of materials representative of those to be used in the actual building construction. The connections of these members shall be consistent with those intended in actual building construction.

6.3.1 For wood framing members, record the species and grade of lumber used; moisture content of lumber at the time of fabrication and testing, if more than 24 h passes between these operations (see Test Methods D 4442, Method A or B; or D 4444, Method A or B); and specific gravity of the lumber (see Test Methods D 2395, Method A).

6.3.2 For steel or other metal framing members, record the material specifications and thickness.

6.4 *Wall Size*—The wall assembly shall have a height and length or aspect (height/length) ratio that is consistent with intended use requirements in actual building construction (see Fig. 3).

7. Test Setup

7.1 The wall assembly shall be tested such that all elements and sheathing surfaces are observable. The bottom of the frame shall be attached to a rigid base with shear connections as specified in 6.2. The test apparatus shall support the wall assembly as necessary to prevent displacement from the plane of the wall, but in-plane displacement shall not be restricted.

8. Procedure

8.1 *Number of Tests*—A minimum of two identical wall assemblies shall be tested to determine the maximum shear stiffness and maximum shear strength of a given construction. These values shall be calculated in accordance with Section 9. If these force-displacement relationships do not agree within 10 % of the lower value, test a third identical wall assembly, and compute the mean value based on the number of walls tested.

8.2 Apply racking shear load horizontally to the top of the wall assembly along the axis of the frame (Fig. 4). A programmable double-acting hydraulic actuator with an integral load cell is suggested for conducting the tests. The cyclic displacement of the actuator shall be controlled to follow a cyclic displacement procedure described in either 8.3 (Method A) or 8.4 (Method B).

8.2.1 The first major event (FME) and the ultimate displacement (Δ_u) shall be determined from a preliminary monotonic load test on an identical wall assembly in accordance with Practice E 564.

8.3 *Method A (Sequential-Phased Displacement Procedure)*:

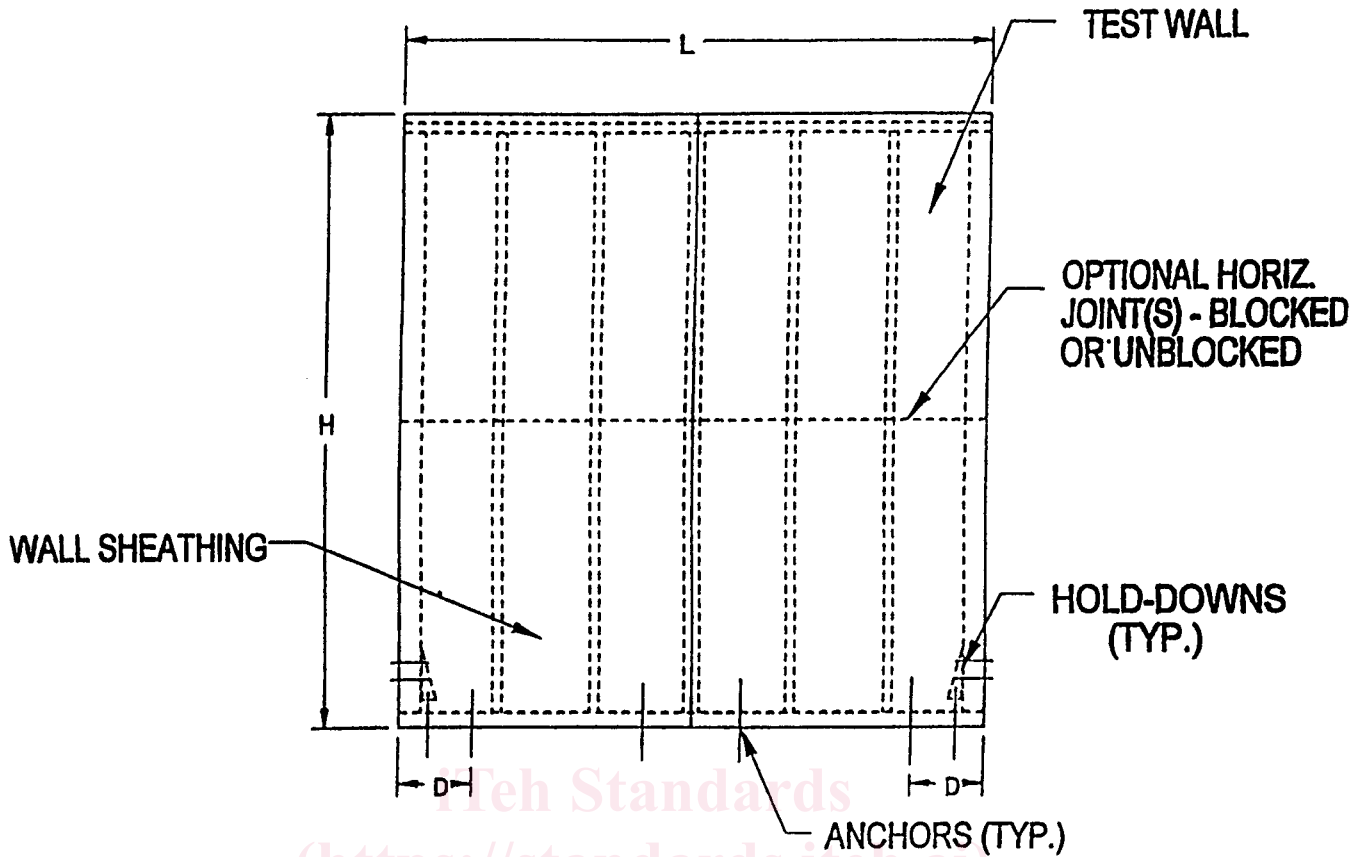


FIG. 3 Cyclic Load Shear Wall Test Specimen

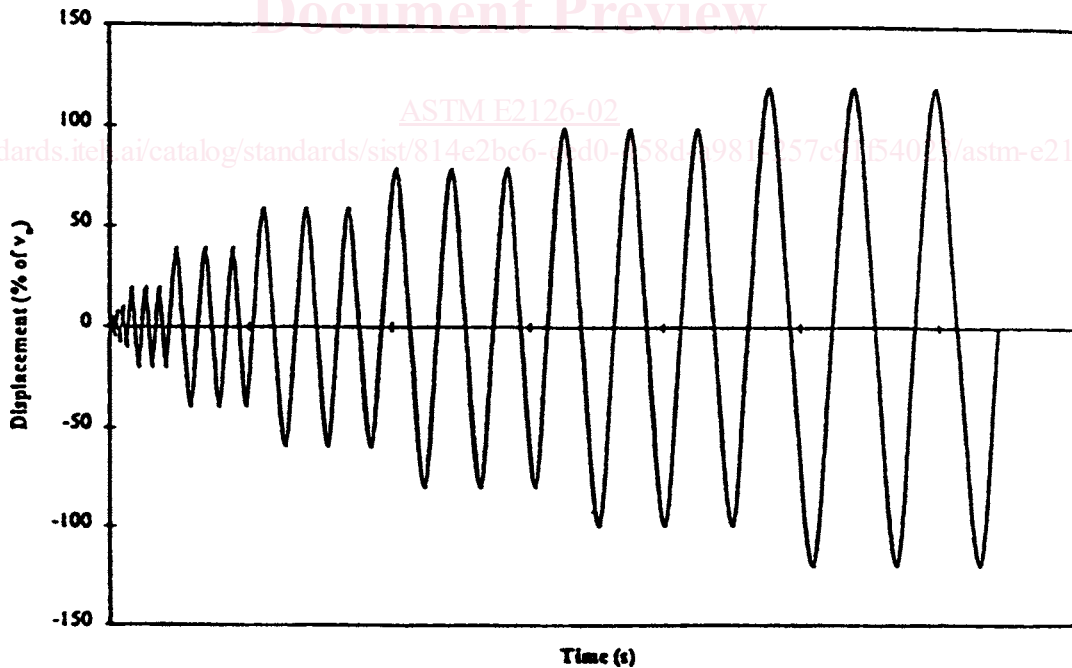


FIG. 4 Cycle Displacement Schedule

8.3.1 *Sequential Phased Displacement (SPD) Loading Procedure*—Displacement-controlled loading procedure that involves displacement cycles grouped in phases at incrementally increasing displacement levels. The cycles shall form

either a sinusoidal wave or a triangular wave. The SPD loading consists of two displacement patterns and is illustrated in Fig. 5. The first displacement pattern consists of three phases, each containing three fully-reversing cycles of equal amplitude, at