

Designation: D 5262 - 02a

Standard Test Method for Evaluating the Unconfined Tension Creep Behavior of Geosynthetics¹

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1. Scope

1.1 This test method is intended for use in determining the unconfined tension creep behavior of geosynthetics at constant temperature when subjected to a sustained tensile loading. This test method is applicable to all geosynthetics.

1.2 The test method measures total elongation of the geosynthetic test specimen, from the time of loading, while being maintained at a constant temperature. It includes procedures for measuring the tension creep behavior at constant temperature of conditioned geosynthetics as well as directions for calculating tension creep curves.

1.3 The values stated in SI units are to be regarded as the standard. The values given in parentheses are for information only.

1.4 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

2. Referenced Documents

2.1 ASTM Standards:

ASTM D

- D 123 Terminology Relating to Textiles² dards/sist/
- D 1776 Practice for Conditioning Textiles for Testing²
- D 1909 Table for Commercial Moisture Regains for Textile Fibers²
- D 2990 Test Methods for Tensile, Compressive, and Flexural Creep and Creep Rupture of Plastics³
- D 4354 Practice for Sampling of Geosynthetics for Testing⁴
- D 4439 Terminology for Geosynthetics⁴
- D 4491 Test Methods for Water Permeability of Geotextiles by Permittivity⁴
- D 4595 Test Method for Tensile Properties of Geotextiles by the Wide-Width Strip Method⁴
- D 4885 Test Method for Determining Performance Tensile

² Annual Book of ASTM Standards, Vol 07.01.

Strength of Geomembranes Using Wide Strip Testing⁴ E 6 Terminology Relating to Methods of Mechanical Testing⁵

3. Terminology

3.1 Definitions—For definitions of many terms used in this test method, refer to Terminologies D 123, D 4439 and E 6.

3.2 Descriptions of Terms Specific to This Standard:

3.2.1 atmosphere for testing geosynthetics, n—air maintained at a relative humidity between 50 and 70% and temperature of $21 \pm 2^{\circ}C$ ($70 \pm 4^{\circ}F$).

3.2.2 creep, *n*—the time-dependent increase in accumulative strain in a material resulting from an applied constant force.

3.2.3 *design load*—the load at which the geosynthetic is required to operate in order to perform its intended function.

3.2.4 *failure*, n—an arbitrary point at which a material ceases to be functionally capable of its intended use.

3.2.5 geogrid, n—a geosynthetic formed by a regular network of integrally connected elements with apertures greater than 6.35 mm (¹/₄ in.) to allow interlocking with surrounding soil, rock, earth, and other surrounding materials to function primarily as reinforcement.

3.2.6 *geomembrane*, *n*—an essentially impermeable geosynthetic composed of one or more synthetic sheets.

3.2.6.1 *Discussion*—In geotechnical engineering, essentially impermeable means that no measurable liquid flows through a geosynthetic when tested in accordance with Test Methods D 4491.

3.2.7 *geosynthetic*, *n*—a planar product manufactured from polymeric material used with soil, rock, earth, or other geotechnical engineering-related material as an integral part of a man-made project, structure, or system.

3.2.8 *geotextile*, *n*—a permeable geosynthetic comprised solely of textiles.

3.2.8.1 *Discussion*—Current manufacturing techniques produce nonwoven fabrics, knitted (non-tubular) fabrics, and woven fabrics.

3.2.9 *index test*, *n*—a test procedure that may contain a known bias, but that may be used to establish an order for a set of specimens with respect to the property being measured.

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³ Annual Book of ASTM Standards, Vol 08.02.

⁴ Annual Book of ASTM Standards, Vol 04.13.

⁵ Annual Book of ASTM Standards, Vol 03.01.

3.2.10 *rate of creep*, *n*—the slope of the creep-time curve at a given time.

3.2.11 *tensile creep rupture strength*, $[FL^{-1}]$, *n*—for geosynthetics, the force per unit width that will produce failure by rupture in a creep test in a given time, at a specified constant environment.

3.2.12 *tensile creep strain*, *n*—the total strain at any given time.

3.2.13 wide strip tensile test, n—for geosynthetics, a tensile test in which the entire width of a 200-mm (8.0 in.)-wide specimen is gripped in the clamps with a gage length of 100 mm (4.0 in.).

4. Summary of Test Method

4.1 The tension creep behavior of geosynthetics is measured by applying a sustained load in one step and measuring the total elongation of the test specimen as a function of time while maintaining a specified temperature and humidity. Unless otherwise stipulated, a temperature of $21 \pm 2^{\circ}$ C ($70 \pm 4^{\circ}$ F) and relative humidity of between 50 and 70 % shall be used.

5. Significance and Use

5.1 This test method is developed for use in the determination of anticipated total elongation that may occur in geosynthetics under sustained loading conditions.

5.1.1 The test data can be used in conjunction with interpretive methods to evaluate creep strain potential at design loads.

5.2 This test method is not intended for routine acceptance testing of geosynthetics. This test method should be used to characterize geosynthetics intended for use in applications in which creep is of concern. The plane strain condition imposed during testing must be considered when using the test results for design.

5.3 The basic distinctions between this test method and other test methods for measuring tension creep behavior are (1) the width of the specimens tested and (2) the measurement of total elongation from the time of specimen loading. The greater widths of the specimens specified in this test method minimize the contraction edge effect (necking) that occurs in many geosynthetic materials and provides a closer relationship to actual material behavior in plane strain tension conditions.

5.4 The creep of a given geosynthetic is likely to be reduced in soil because of load transfer to the soil. The unconfined environment represents a controlled test, in which the results are conservative with regard to the behavior of the material in service. Confined or in-soil testing may model the field behavior of the geosynthetic more accurately.

6. Apparatus

6.1 Clamps:

6.1.1 Clamps should be at least as wide as the specimen, with appropriate clamping power that will prevent slipping or damage of the test specimen within or at the faces of the clamps. The clamps and clamping technique shall be designed to minimize eccentric loading of the specimen. A swivel or universal joint shall be used on one of the clamps at the end of

the specimen. It is recommended that clamps permit the final centering of the specimen prior to application of the load.⁶

6.1.2 *Geotextiles and Geomembranes*—Each clamp shall be sufficiently wide to grip the entire width of the specimen, 200 mm (8.0 in.), and a minimum of 50-mm (2.0-in.) length in the direction of the applied force.

6.1.3 *Geogrids*—These should be clamped to assure complete tension load transfer through test direction members. The type of clamp and load transfer mechanism should be detailed in the test report. Roller grips or low melting point alloy with adequate strength may be used to assist proper clamping.

6.1.4 *Other Related Products*—Where special clamps are used to grip these products, they should conform to the general requirements for clamps used to grip geotextiles, geomembranes, and geogrids, and the clamping methods used should always be detailed in the report.

6.2 Loading System—The loading system must be designed so that the load applied and maintained on the specimen is within ± 1 % of the desired load. Loads may be applied by weights, weights and fulcrums, or pneumatics. The loading mechanism must permit reproducibly rapid and smooth loading, as specified in 11.4. No dynamic forces on placement of the loads shall be allowed. Provision must also be made to ensure that shock loading, caused by specimen failure, is not transferred to other specimens undergoing testing.⁶

6.3 *Extension Measurement*—Extensioneters are preferred for the measurement of elongation in geosynthetics. Whenever possible, other means of measuring elongation should be calibrated against extensioneters. In any case, the device chosen shall be capable of measuring deformations to an accuracy of at least $0.003 \pm \text{mm} (0.0001 \pm \text{in.})$. The means of measuring elongation should be indicated clearly in the report.

6.4 Vibration Control—Creep tests are sensitive to shock and vibration. The location of the apparatus, test equipment, and mounting shall be designed so that the specimen is isolated from vibration. Multi-station test equipment must be of sufficient rigidity so that no significant deflection due to shock or vibration occurs during creep testing.

6.5 *Time Measurement*—The accuracy of the time measuring device shall be ± 1 % of the elapsed time of each creep measurement load increment.

6.6 Temperature Control and Measurement:

6.6.1 The temperature in the test space, especially close to the gage length of the specimen, shall be maintained within $\pm 2^{\circ}C$ ($\pm 4^{\circ}F$) of the targeted value by a suitable automatic device and shall be stated in the report. It is generally recognized that thermal contraction and expansion, associated with small temperature changes during the test, may produce changes in the apparent creep rate, especially near the transition temperature.

6.6.2 Temperature measurements shall be recorded at frequent intervals, or recorded continuously, in order to ensure an accurate determination of the average test temperature and compliance with 6.6.1.

6.7 Environmental Control and Measurement:

⁶ Examples of clamping, loading, and extensometer systems that have been used successfully are found in the appendixes.

6.7.1 When the test environment is air, the relative humidity shall be maintained between 50 and 70 % unless the creep behavior of the geosynthetic has been shown to be unaffected by humidity. The relative humidity shall be recorded at frequent intervals to ensure that an accurate determination of the average test humidity can be made.

6.7.2 The test environment shall be maintained constant throughout the test. Safety precautions should be taken to avoid personal contact during the test. The area should be isolated adequately and fenced such that only the test operator has access to the test station.

7. Sampling

7.1 *Laboratory Sample*—For the laboratory sample, take a full-width swatch approximately 1-m (40-in.) long in the machine direction from each roll in the lot sample. The sample may be taken from the end portion of a roll, provided there is no evidence that it is different from other portions of the roll.

7.2 Test Specimens:

7.2.1 *Geotextiles and Geomembranes*—For tests in the machine and cross-machine directions, respectively, take from each sample the number of specimens as directed in 9.1. Take the specimens from a diagonal on the sample, with no specimens closer than ¹/₁₀ the width of the roll or 150 mm (6 in.), whichever is smaller. For geomembranes, exercise care in selecting, cutting, and preparing the specimens to avoid nicks, tears, scratches, folds, or other imperfections that are likely to cause premature failure.

NOTE 1—Nonreinforced geomembranes are extremely sensitive in this regard.

7.2.2 Geogrids and other Related Products—For tests in the machine and cross-machine directions, respectively, take from each sample the number of specimens as directed in 9.19.1. Take specimens at random from the laboratory sample. For the measurement of machine direction properties, take specimens from different positions across the width of the sample.For the measurement of cross-machine direction properties, take specimens from different positions along the length of the sample. Take no specimens nearer to the edge than $\frac{1}{10}$ the width of the roll or 150 mm (6 in.), whichever is smaller.

8. Test Specimen

8.1 *Geosynthetics*—Prepare each finished specimen to the width appropriate for the particular geosynthetic with the length dimension parallel to the direction that the creep or creep rupture behavior is being measured.

8.1.1 *Geotextiles*—Prepare specimen width to 200 mm (8.0 in) wide by at least 200 mm (8.0 in) long.

8.1.2 *Geogrid*—Prepare specimen width to include at least three longitudinal elements abreast parallel to the direction that the creep or creep rupture behavior is being measured with each element long enough to include at least three aperatures, as illustrated in Fig. X2.1.

8.2 The length of the specimen depends on the type of clamps being used. The specimen must be long enough to extend through the full length of both clamps, as determined for the direction of the test.

8.3 When specimen integrity is not affected, the specimen may be cut initially to the finished width.

8.4 This test method may not be suitable for some woven geotextiles or geogrids that exhibit breaking strengths in excess of 100 kN/m (600 lbf/in.), due to clamping and equipment limitations.

9. Number of Tests

9.1 Unless otherwise agreed upon, creep tests shall be conducted at load levels as specified by the designer. Four load levels are recommended for characterization of the material. Loads shall be selected at intervals of approximately 10 % of the maximum load per unit width, that is, 20, 30, 40, and 60 %, as determined by applicable ASTM test methods.

NOTE 2—It is generally recognized that characterization involves identification of the load levels at which there is no creep (no increase in strain with the log of time), low to moderate creep (linear increase in strain with the log of time), and high creep (exponential increase in strain with the log of time).

9.2 To evaluate design creep strains, it is recommended that a minimum of two creep tests be performed (that is, one at the design load and one at a load that exceeds the design load, as specified by the designer).

10. Conditioning and Testing Atmosphere

10.1 Bring the specimens to moisture equilibrium in the atmosphere for testing geosynthetics. Equilibrium is considered to have been reached when the increase in mass of the specimen, in successive weighings made at intervals of not less than 2 h, does not exceed 0.1 % of the specimen mass. In general practice, the industry approaches equilibrium from the as-received side.

Note 3—It is customary that geosynthetic materials are frequently not weighed to determine when moisture equilibrium has been reached. While such a procedure cannot be accepted in cases of dispute, in routine testing, it may be sufficient to expose the material to the standard atmosphere for testing for a reasonable time period before the specimens are tested. A time period of 24 h has been found acceptable in most cases. However, certain fibers may exhibit slow moisture equilibrium rates from the as-received wet side. When this is known, a preconditioning cycle, as prescribed in Practice D 1776, may be agreed upon between contractural parties.

10.2 To characterize the influence of temperature, in addition to the standard temperature of $21 \pm 2^{\circ}$ C ($70 \pm 4^{\circ}$ F), two or more additional temperatures may be used to cover the useful temperature range of the geosynthetic considered. These should be chosen in suitable increments reflecting the variation of creep of the geosynthetic with temperature and transitions of the material. The test temperature will generally be determined by site conditions and should be agreed upon by contractual parties. Recommended additional temperatures are $10 \pm 2^{\circ}$ C ($50 \pm 4^{\circ}$ F) and $40 \pm 2^{\circ}$ C ($104 \pm 4^{\circ}$ F). In any case, the temperature or temperatures chosen for testing should be recorded in the report.

11. Procedure

11.1 Test adequately conditioned specimens. Conduct the tests at a temperature of $21 \pm 2^{\circ}C$ (70 $\pm 4^{\circ}F$) and relative

humidity of 50 to 75 %. The engineer may specify additional temperatures based on expected service conditions for the installation.

11.2 Mount the specimen centrally in the clamps. Achieve this by having the two lines, which were previously drawn, 100 ± 3 -mm (4.0 \pm 0.1-in.) apart across the width of the specimen positioned adjacent to the inside edges of the upper and lower jaw. The specimen length in the machine direction and cross machine direction tests, respectively, must be parallel to the direction of application of force.

11.3 Attach the extension measuring devices directly to the specimen. If these are optical devices, set up the measurement mechanism accordingly. Make the initial or reference measurement. It is recommended that the initial gage length be set at 75 mm (3 in.) for geotextiles and geomembranes. The initial gage length for geogrids should be centered between nodes and contain a minimum of one node.

11.4 Where required, place a pretension force, which includes any load due to the mass of the clamps, rapidly and smoothly on the specimen. In any case, the total time between placement of a pretension load and full load shall not exceed 10 min. Record the pretension force and resulting extension. It is generally accepted that the application of a pretension force is required when testing certain geosynthetics, for which part of the extension on loading occurs from a realignment of fiber structure and is relatively variable, while the subsequent time-dependent elongation, which is due to creep of the fibers, is more consistent. The application of a pretension force has therefore been selected as a simple means of establishing zero strain.

11.5 The pretension force, which includes the weight of the loading mechanism and weight of grip, should have a maximum total applied force on the specimen of 45 N (10 lbf) for materials exhibiting a breaking force of 17 500 N/m (100 lbf/in.) and under, as determined in accordance with Test Method D 4595. For materials exhibiting a breaking force in excess of 17 500 N/m (100 lbf/in.), a pretension force equal to 1.25 % of the expected breaking force should be applied: however, in no case should the total pretension force exceed 300 N (67.5 lbf).

11.6 Apply the full load rapidly and smoothly to the specimen, preferably at a strain rate of 10 ± 3 %/min. Record the total time for loading (excluding pretension). In any event, disregard measurements within five times of the loading time.

11.7 Measure the extension of the specimen in accordance with the following approximate time schedule: 1, 2, 6, 10, and 30 min; and 1, 2, 5, 10, 30, 100, 200, 500, and 1000 h. For creep tests longer than 1000 h, measure and record extension every 500 h until testing is complete.

NOTE 4—In design, it is generally accepted that creep data should not be extrapolated beyond one order of magnitude. In many cases, a test period of 1000 h therefore, may not reflect the long-term creep behavior of the material accurately. For such cases, creep tests should be conducted for a minimum of 10 000 h.

NOTE 5—For preliminary evaluation of newly developed products when creep testing is underway but has not yet reached 10 000 h, creep behavior may be inferred from completed test results on essentially identical products from the same family of products; (that is, manufactured by the same organization using the same process technology, polymer type, polymer structure, polymer molecular weight, polymer additives, constituent materials, product configuration, etc.). Application of this inference is appropriate only when a minimum of 1000 h of creep testing is completed on the new product and a definable correlation exists with the available 10 000 h creep test results for the family of products which bound the ultimate strength and constant load level for creep testing of the new product.

11.8 Readings should be recorded more frequently if discontinuities in the creep strain versus log of time plot are suspected or encountered. To avoid such discontinuities, the use of automatic monitoring and measuring equipment is recommended.

11.9 Terminate a test when the specimen ruptures or at the end of the agreed upon period. If the specimen ruptures, report the type of failure, location, and time to failure.

12. Calculation

12.1 *Creep Curves*—The standard curve is a graph of strain versus log of time, as shown in Fig. 1.The data are prepared by use of the following calculations:

12.1.1 *Time*—Elapsed time intervals are converted to hours and converted to the log of time (in hours).

12.1.2 *Strain*—The percent strain at each interval is calculated (to the nearest 0.1 mm) using Eq 1:

$$\boldsymbol{\epsilon} = \left(\Delta L \times 100\right) / L_g \tag{1}$$

where:

 $\epsilon = \text{strain}, \%,$

- ΔL = unit change in length from the pretension force to the corresponding applied force, mm (in.), and
- L_g = initial nominal gage length plus the pretension displacement, mm (in.).

12.1.3 The data are then plotted as percent strain as ordinate versus log of time as abscissa. If several loads are used for testing, each plot shall be labeled clearly with the appropriate loading or force per unit width, expressed in kN/m (lb/in.). For geogrids, the equivalent force per unit width is determined by the use of Eq 2:

$$a = (F / N_R) \times N_T \tag{2}$$

where:

a = equivalent force per unit width, kN/m (lbf/ft),

F = applied force, kN (lbf),

 N_R = number of ribs tested, and

 N_T = number of ribs per unit width.

