



**SLOVENSKI STANDARD**  
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**Nadzemni rezervoarji in posode iz umetnih mas, ojačanih s steklenimi vlakni - 5.**  
**del: Primer izračuna**

GRP tanks and vessels for use above ground - Part 5: Example calculation of a GRP-vessel

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Nepremične posode in  
rezervoarji

Stationary containers and  
tanks

**kSIST-TP FprCEN/TR 13121-5:2017**

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**GRP tanks and vessels for use above ground - Part 5:**  
**Example calculation of a GRP-vessel**

This draft Technical Report is submitted to CEN members for Vote. It has been drawn up by the Technical Committee CEN/TC 210.

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EUROPEAN COMMITTEE FOR STANDARDIZATION  
COMITÉ EUROPÉEN DE NORMALISATION  
EUROPÄISCHES KOMITEE FÜR NORMUNG

**CEN-CENELEC Management Centre: Avenue Marnix 17, B-1000 Brussels**

<b>Contents</b>	<b>Page</b>
European foreword.....	5
Introduction .....	6
1 Scope.....	7
2 General.....	7
3 Dimensions of the tank .....	7
4 Building materials .....	9
5 Loadings (9) .....	9
6 Limit strain for laminate (8.2.2) .....	11
7 Influence factors (7.9.5.2) .....	11
8 Partial safety factors (Table 12) .....	12
9 Combination factors (Table 11).....	12
10 Analysis of the cylinder.....	12
10.1 Influence factor $A_5$ .....	13
10.2 Characteristic strength values.....	13
10.3 Moduli of elasticity .....	13
10.4 Analysis of the cylinder in axial direction.....	13
10.4.1 Proof of strength (Ultimate limit state) .....	14
10.4.2 Proof of strain (Serviceability limit state) .....	17
10.4.3 Stability proof (Ultimate limit state).....	19
10.5 Analysis of the cylinder in tangential direction.....	21
10.5.1 Strength analysis (Ultimate limit state).....	21
10.5.2 Proof of strain (Serviceability limit state) .....	23
10.5.3 Stability proof for the cylindrical shell tangential (Ultimate limit state).....	23
10.5.4 Critical buckling pressure for rings (Ultimate limit state) .....	24
10.6 Earthquake design of the cylinder .....	26
10.6.1 Analysis of the cylinder in axial direction.....	26
10.6.2 Analysis of the cylinder in tangential direction .....	29
11 Opening in the cylinder.....	30
11.1 Analysis in circumferential direction .....	31
11.1.1 Proof of strength.....	31
11.1.2 Proof of strain.....	31
11.2 Analysis in axial direction.....	32
11.2.1 Proof of strength.....	32
11.2.2 Proof of strain.....	32
12 Analysis of the skirt.....	32
12.1 Internal forces of the skirt.....	33
12.2 Proof of strength (Ultimate limit state) .....	34
12.2.1 Design value of actions.....	34
12.2.2 Design value of corresponding resistance .....	34
12.2.3 Verification.....	35
12.3 Proof of strain (Serviceability limit state) .....	35
12.3.1 Design value of actions.....	35

12.3.2	Limit design value of serviceability criterion.....	35
12.3.3	Verification .....	35
12.4	Stability proof (Ultimate limit state) .....	36
12.4.1	Design value of actions .....	36
12.4.2	Design value of corresponding resistance.....	36
12.4.3	Verification .....	36
12.5	Earthquake design of the skirt.....	37
12.5.1	Internal forces Earthquake .....	37
12.5.2	Proof of strength (Ultimate limit state).....	37
12.5.3	Proof of strain (Serviceability limit state).....	38
12.5.4	Stability proof (Ultimate limit state) .....	39
13	Overlay laminate connection skirt - vessel .....	39
13.1	Proof of strength (Ultimate limit state).....	39
13.1.1	Design value of actions .....	39
13.1.2	Design value of corresponding resistance.....	40
13.1.3	Verification .....	40
13.2	Proof of strain (Serviceability limit state).....	40
13.2.1	Design value of actions .....	40
13.2.2	Limit design value of serviceability criterion.....	40
13.2.3	Verification .....	41
13.3	Seismic design of the skirt overlay .....	41
13.3.1	Proof of strength (Ultimate limit state).....	41
13.3.2	Proof of strain (Serviceability limit state).....	41
14	Analysis of the bottom .....	42
14.1	Influence factor $A_5$ .....	42
14.2	Characteristic strength values .....	42
14.3	Moduli of elasticity .....	42
14.4	Actions, which cause internal forces for the bottom .....	42
14.5	Strength analysis (Ultimate limit state) .....	42
14.5.1	Design value of actions .....	42
14.5.2	Proof of strain (Serviceability limit state).....	44
14.5.3	Stability proof of the bottom (Ultimate limit state) .....	45
15	Lower part of the cylinder (Region 1).....	46
15.1	Strength analysis (Ultimate limit state) .....	46
15.1.1	Design value of corresponding resistance.....	47
15.1.2	Verification .....	47
15.2	Proof of strain (Serviceability limit state).....	47
15.2.1	Design value of actions .....	47
15.2.2	Limit design value of serviceability criterion.....	47
15.2.3	Verification .....	48
15.3	Earthquake design of region 1 (Ultimate limit state).....	48
15.3.1	Strength analysis (Ultimate limit state) .....	48
15.3.2	Proof of strain (Serviceability limit state).....	49
16	Upper part of the skirt (Region 2) .....	49
16.1	Strength analysis (Ultimate limit state) .....	49
16.1.1	Design value of corresponding resistance.....	50
16.1.2	Verification .....	50
16.2	Proof of strain (Serviceability limit state).....	50
16.2.1	Design value of actions .....	50
16.2.2	Limit design value of serviceability criterion.....	50
16.2.3	Verification .....	51

## FprCEN/TR 13121-5:2016 (E)

16.3	Seismic design of region 2 (Ultimate limit state) .....	51
16.3.1	Strength analysis (Ultimate limit state).....	51
16.3.2	Design value of corresponding resistance .....	51
16.3.3	Verification.....	51
16.4	Proof of strain (Serviceability limit state) .....	51
16.4.1	Design value of actions.....	51
16.4.2	Limit design value of serviceability criterion .....	52
16.4.3	Verification.....	52
17	Flange design .....	52
18	Anchorage .....	57
18.1	Anchorage for wind loads (Permanent / Transient situation).....	57
18.1.1	Uplifting anchor force.....	57
18.1.2	Anchor shear force.....	57
18.2	Anchorage for seismic loads (Seismic design situation).....	57
18.2.1	Uplifting anchor force.....	57
18.2.2	Anchor shear force.....	58

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## European foreword

This document (FprCEN/TR 13121-5:2016) has been prepared by Technical Committee CEN/TC 210 “GRP tanks and vessels”, the secretariat of which is held by SFS.

This document is currently submitted to the Vote on TR.

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## Introduction

EN 13121 consists of the following parts:

- EN 13121-1, *GRP tanks and vessels for use above ground — Part 1: Raw materials — Specification and acceptance conditions*
- EN 13121-2, *GRP tanks and vessels for use above ground — Part 2: Composite materials — Chemical resistance*
- EN 13121-3, *GRP tanks and vessels for use above ground — Part 3: Design and workmanship*
- EN 13121-4, *GRP tanks and vessels for use above ground — Part 4: Delivery, installation and maintenance*
- CEN/TR 13121-5, *GRP tanks and vessels for use above ground — Part 5: Example calculation of a GRP-tank (this report)*

These five parts together define the responsibilities of the tank or vessel manufacturer and the materials to be used in their manufacture.

EN 13121-1 specifies the requirements and acceptance conditions for the raw materials - resins, curing agents, thermoplastics linings, reinforcing materials and additives. These requirements are necessary in order to establish the chemical resistance properties determined in EN 13121-2 and the mechanical, thermal and design properties determined in EN 13121-3. Together with the workmanship principles determined in Part 3, requirements and acceptance conditions for raw materials ensure that the tank or vessel will be able to meet its design requirements. EN 13121-4 of this standard specifies recommendations for delivery, handling, installation and maintenance of GRP tanks and vessels. This part of EN 13121 gives guidance in use of the standard. CEN/TC 210 has found it necessary to publish an example calculation of a vessel according to EN 13121-3 due to the standards complexity, and for the understanding of how the standard complies with EN 1990:s principles and requirements for safety, serviceability and durability of structures.

The design and manufacture of GRP tanks and vessels involve a number of different materials such as resins, thermoplastics and reinforcing fibres and a number of different manufacturing methods. It is implicit that vessels and tanks covered by this standard are made only by manufacturers who are competent and suitably equipped to comply with all the requirements of this standard, using materials manufactured by competent and experienced material manufacturers.

Metallic vessels, and those manufactured from other isotropic, homogeneous materials, are conveniently designed by calculating permissible loads based on measured tensile and ductility properties. GRP, on the other hand, is a laminar material, manufactured through the successive application of individual layers of reinforcement. As a result there are many possible combinations of reinforcement type that will meet the structural requirement of any one-design case. This allows the designer to select the laminate construction best suited to the available manufacturing facilities and hence be most cost effective.



## 1 Scope

This Technical Report gives guidance for the design of a vessel using the standard EN 13121-3 GRP tanks and vessels for use above ground. The calculation is done according to the advanced design method given in EN 13121-3:2016, 7.9.3 with approved laminates and laminate properties.

## 2 General

Vessels or vessel structures may contain such structural elements or solutions for which this standard does not provide sufficient guidance. In that case, other methods shall be used in order to obtain a safe structure.

This example calculation is based on a pressurized GRP vessel with an internal diameter of D 3000 mm. The cylindrical parts of the vessel are filament wound. Its bottom and roof are torispherical dished ends that are hand laid up using mixed laminates. Protection against medium attack is obtained by a chemical resistance layer (CRL).

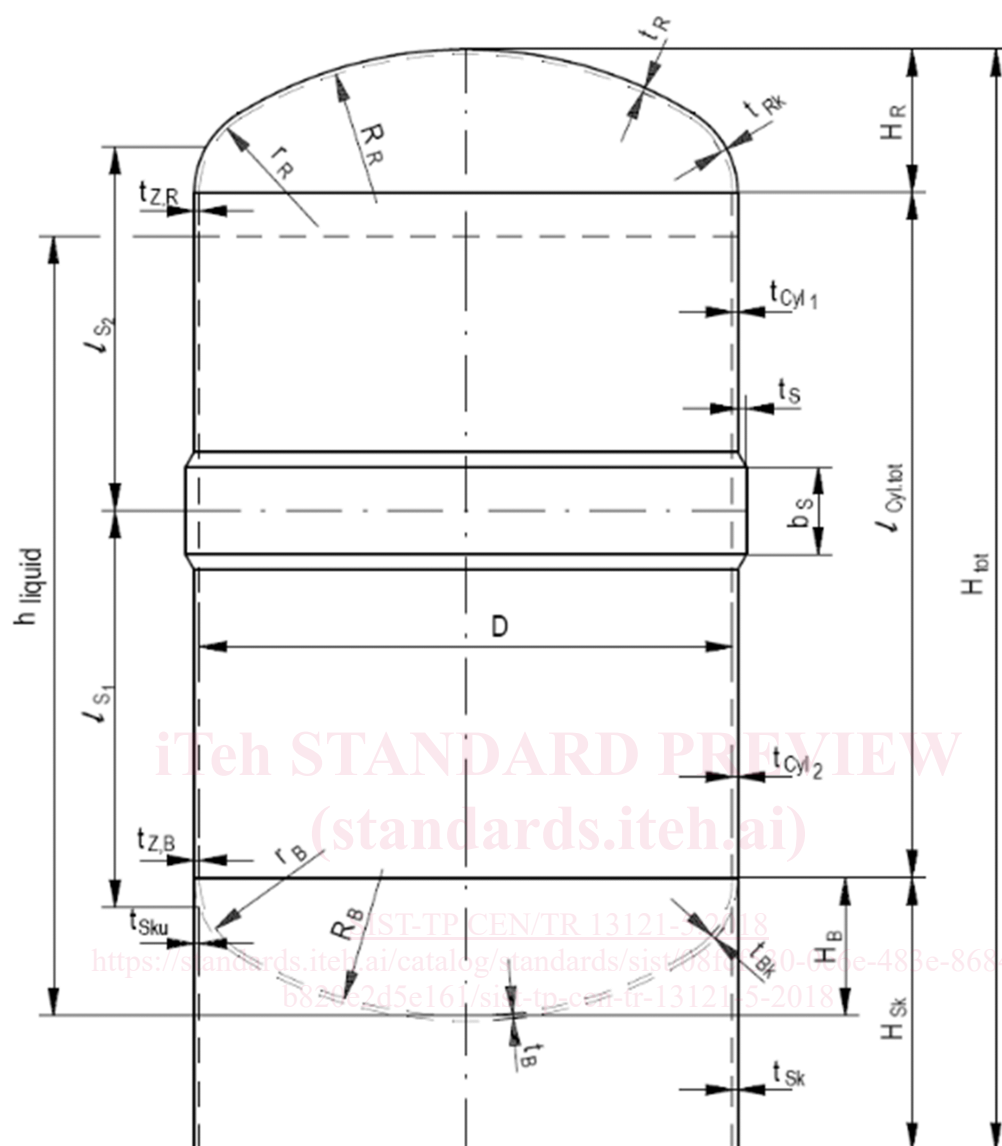
The tank is located outdoors in a seismic area.

IMPORTANT – This example doesn't cover all necessary verifications for the calculation of the GRP tank. Additional verifications have to be performed for the roof, the upper cylinder, etc.

## 3 Dimensions of the tank

Sketch of the tank dimensions:

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**General Dimensions:**

Diameter:  $D = 3\,000\text{ mm}$

Total height:  $H_{\text{tot}} = 8\,000\text{ mm}$

**Cylinder:**

Thickness cylinder 1:  $t_{\text{Cyl},1} = t_{\text{C}1} = 9,2\text{ mm}$

Thickness cylinder 2:  $t_{\text{Cyl},2} = t_{\text{C}2} = 11,7\text{ mm}$

Thickness cylinder at roof:  $t_{\text{Z},\text{R}} = 30,0\text{ mm}$

Thickness cylinder at bottom:  $t_{\text{Z},\text{B}} = 46,1\text{ mm}$

Total cylinder length:  $l_{\text{Cyl,tot}} = 6\,610\text{ mm}$

Distance between stiffeners:  $l_{\text{s},1} = 3\,700\text{ mm}$   $l_{\text{s},2} = 3\,303\text{ mm}$

Thickness of the stiffener:  $t_{\text{s}} = 20\text{ mm}$

Width of the stiffener:  $b_s = 260 \text{ mm}$

**Skirt:**

Thickness skirt:  $t_{sk} = 17,0 \text{ mm}$

Thickness overlay laminate:  $t_{02} = 7,0 \text{ mm}$

Height of the skirt:  $H_{sk} = 890 \text{ mm}$

**Roof:**

Thickness calotte:  $t_R = 13,0 \text{ mm}$

Radius calotte:  $R_R = 3000 \text{ mm}$

Thickness knuckle roof:  $t_{Rk} = 30,0 \text{ mm}$

Radius knuckle:  $r_{Rk} = 300 \text{ mm}$

Height of the roof:  $H_R = 590 \text{ mm}$

**Bottom:**

Thickness calotte:  $t_B = 16,5 \text{ mm}$

Radius calotte:  $R_B = 3000 \text{ mm}$

Thickness knuckle:  $t_{Bk} = 45,0 \text{ mm}$

Radius knuckle:  $r_{Bk} = 300 \text{ mm}$

Height of the bottom:  $H_B = 590 \text{ mm}$

## 4 Building materials

Resin type: UP-resin, Resin group 4

## 5 Loadings (9)

### LC 1: Dead load

The assumed dead loads for the separate tank parts are:

Roof:  $W_{R,k} = 4 \text{ kN}$  Area load:  $w_{R,k} = 0,57 \text{ kN/m}^2$

Cylinder + rings:  $W_{C,k} = 19 \text{ kN}$

Bottom:  $W_{B,k} = 4 \text{ kN}$  Area load:  $w_{B,k} = 0,57 \text{ kN/m}^2$

Skirt:  $W_{sk} = 3 \text{ kN}$

Total dead load of the vessel:  $W_{tot} = 30 \text{ kN}$

### LC 2: Liquid filling

Density of the medium  $\rho_{liquid} = 1,30 \text{ kg/dm}^3$

Filling height  $h_{liquid} = 7000 \text{ mm}$

Volume  $V = 52,0 \text{ m}^3$

### LC 3: Long time design overpressure

Design pressure

$PS_{op,l} = 2,000 \text{ bar} \equiv 0,20 \text{ N/mm}^2$

### LC 4: Short time design overpressure

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Design pressure  $PS_{op.s} = 2,500 \text{ bar} \equiv 0,25 \text{ N/mm}^2$

LC 5: Long time design negative pressure

Design pressure  $PS_{ep.l} = 0,000 \text{ bar} \equiv 0,00 \text{ N/mm}^2$

LC 6: Short time design negative pressure

Design pressure  $PS_{ep.s} = 0,050 \text{ bar} \equiv 0,005 \text{ N/mm}^2$

LC 7: Wind (9.2.2)

Peak velocity pressure  $q_p = 0,8 \text{ kN/m}^2$  (EN 1991-1-4)

Force coefficient (cylindrical vessel)  $c_f = 0,8$

External pressure arising from wind load:  $p_{wind} = 0,6 \cdot q_p = 0,6 \cdot 0,8 = 0,48 \text{ kN/m}^2$

LC 8: Snow (9.2.1)

Characteristic snow load  $s_k = 0,85 \text{ kN/m}^2$  (EN 1991-1-3)

Shape coefficient  $\mu = 0,80$

Snow load  $p_{snow} = s_k \cdot \mu = 0,85 \cdot 0,8 = 0,68 \text{ kN/m}^2$

LC 9: Personnel loading (9.2.8)

Live load on the roof  $p_{access} = 1,5 \text{ kN/m}^2$

LC 10: Temperature

Design temperature  $TS = 50^\circ\text{C}$

Difference in temperature  $\Delta T = 20 \text{ K}$

LC 11: Earthquake (9.2.3.4)

Reference peak ground acceleration  $a_{gR} = 1,00 \text{ m/s}^2$

Importance factor  $\gamma_1 = 1,4$

Design ground acceleration  $a_g = a_{gR} \cdot \gamma_1 = 1,00 \cdot 1,4 = 1,40 \text{ m/s}^2$

Ground type according to EN 1998-1 D

Viscous damping 5 %

Control periods of the response spectrum  $T_B = 0,20 \text{ s}$   $T_C = 0,8 \text{ s}$   $T_D = 2,0 \text{ s}$

Soil factor  $S = 1,35$

Behaviour factor  $q = 1,5$

Bending modulus cylinder tangential  $E_{\phi,b} = 19\,000 \text{ N/mm}^2$

Bending modulus cylinder axial  $E_{x,b} = 12\,000 \text{ N/mm}^2$

Modulus of elasticity for short time impact  $E_e = 1,5 \cdot \sqrt{E_{\phi,b} \cdot E_{x,b}} = 1,5 \cdot \sqrt{19\,000 \cdot 12\,000} = 22\,650 \text{ N/mm}^2$

Cylinder thickness lower third  $t_{1/2}$  approximately  $t_{sk} = 17 \text{ mm}$

Vibration period

$$T = \sqrt{\frac{\rho_{liquid} \cdot h_{liquid}}{E_e \cdot t_{1/2}}} \cdot D \cdot \left[ 0,628 \cdot \left( \frac{h_{liquid}}{D} \right)^2 + \frac{2 \cdot h_{liquid}}{D} + 1,49 \right]$$

$$T = \sqrt{\frac{1,33 \cdot 7200}{22650 \cdot 17,0 \cdot 10^3}} \cdot 3,0 \cdot \left[ 0,628 \cdot \left( \frac{7200}{3000} \right)^2 + \frac{2 \cdot 7200}{3000} + 1,49 \right] = 0,15 \text{ s}$$

Design spectrum  $T \leq T_c$   
(plateau area):

$$S_D(T) = a_g \cdot S \cdot \frac{2,5}{q} = 1,40 \cdot 1,35 \cdot \frac{2,5}{1,5} = 3,15 \text{ m/s}^2$$

Total mass of the vessel  
(approximately)

$$W_G = \frac{W_{tot}}{g} + V \cdot \rho_{liquid} = \frac{30}{9,81} + 52 \cdot 1,30 = 70,66 \text{ Tonnen}$$

Horizontal load (Base shear)

$$H_{AE} \cong S_D(T) \cdot W_G = 3,15 \cdot 70,66 = 222,6 \text{ kN}$$

Overturning moment

$$M_{AE,tot} \cong \left[ V \cdot \rho_{liquid} \cdot \left( \frac{h_{liquid}}{2} + H_{Sk} - H_B \right) + \frac{W_{tot}}{g} \cdot \frac{H_{tot}}{2} \right] \cdot S_D(T)$$

$$M_{AE,tot} \cong \left[ 52 \cdot 1,30 \cdot \left( \frac{7\,000}{2} + 800 - 590 \right) \cdot 10^{-3} + \frac{30}{9,81} \cdot \frac{8\,000}{2} \cdot 10^{-3} \right] \cdot 3,15 = 828,5 \text{ kNm}$$

## 6 Limit strain for laminate (8.2.2)

For the used UP resin is:

The roof is made of a mixed laminate  $\varepsilon_{lim,R} = \varepsilon_{d,R} = 0,25 \%$

The bottom is made of a mixed laminate  $\varepsilon_{lim,B} = \varepsilon_{d,B} = 0,25 \%$

The cylinder is made of a wound laminate  $0^\circ$   $\varepsilon_{lim,x,Cyl} = \varepsilon_{d,x,Cyl} = 0,20 \%$   $\varepsilon_{lim,\phi,Cyl} = \varepsilon_{d,\phi,Cyl} = 0,27 \%$   
/90°

The skirt is made of a wound laminate  $0^\circ$  /90°  $\varepsilon_{lim,x,Sk} = \varepsilon_{d,x,Sk} = 0,20 \%$   $\varepsilon_{lim,\phi,Sk} = \varepsilon_{d,\phi,Sk} = 0,27 \%$

## 7 Influence factors (7.9.5.2)

Influence factor  $A_1$   $A_1 = 1,0$

Influence factor  $A_2$   $A_2 = 1,4$  (Table A.4 of EN 13121-2)

Medium category 2,  $T_d = 50^\circ\text{C}$

HDT of the used resin  $\text{HDT} = 90^\circ\text{C}$

Influence factor  $A_3$   $A_3 = 1,00 + 0,4 \cdot \left( \frac{TS - 20^\circ\text{C}}{\text{HDT} - 30^\circ\text{C}} \right) = 1,00 + 0,4 \cdot \left( \frac{50 - 20}{90 - 30} \right) = 1,20$

Influence factor  $A_4$   $A_4 = 1,0$

Influence factor  $A_5$  The influence factor  $A_5$  depends on laminate type and is selected separately for each kind of laminate.

## 8 Partial safety factors (Table 12)

Action	Symbol	Situation	
		P/T	A/AE
<u>Independent permanent actions (s.a):</u>			
unfavourable	$\gamma_{G,sup}$	1,35	1,00
favourable	$\gamma_{G,inf}$	1,00	1,00
For liquid filling			
unfavourable	$\gamma_{G,sup}$	1,35	1,00
favourable	$\gamma_{G,inf}$	0	0
<u>Independent variable actions:</u>			
unfavourable	$\gamma_{Q,sup}$	1,50	1,00
favourable	$\gamma_{Q,inf}$	0	0
<u>Accidental actions:</u>	$\gamma_A$		1,00
<u>Seismic actions:</u>	$\gamma_{AE}$		1,00

## 9 Combination factors (Table 11)

In the following table are shown the relevant  $\Psi$ -factors for this example.

Action	$\psi_0$	$\psi_1$	$\psi_2$
Pressures:			
- Long term pressures	1,0	1,0	1,0
- Short-term pressures	0	0	0
Imposed loads in buildings, category (see EN 1991-1-1)			
- Category H: roofs	0	0	0
Snow loads on buildings (see EN 1991-1-3) <sup>a)</sup> :	0,5	0,2	0
Remainder of CEN Member States, - for sites located at altitude $H \leq 1000$ m a.s.l.			
Wind loads on buildings (see EN 1991-1-4)	0,6	0,2	0
Temperature (non-fire) in buildings (see EN 1991-1-5)	0,6	0,5	0

## 10 Analysis of the cylinder

The cylinder is made of a wound laminate  $0^\circ / 90^\circ$ . For mechanical properties are used historic test data. They are verified with tests in accordance to 7.9.3.

### 10.1 Influence factor $A_5$

#### — For stress analysis

25 years:	a)	Axial	$A_{5B.Cyl.25y.x} = 1,60$	b) Tangential	$A_{5B.Cyl.25y.\phi} = 1,20$
3 months:			$A_{5B.Cyl.3m.x} = 1,40$		$A_{5B.Cyl.3m.\phi} = 1,15$
Shorttime:			$A_{5B.Cyl.sh.x} = 1,00$		$A_{5B.Cyl.sh.\phi} = 1,00$

#### — For stability analysis

25 years:	a)	Axial	$A_{5I.Cyl.25y.x} = 1,60$	b) Tangential	$A_{5I.Cyl.25y.\phi} = 1,20$
3 months:			$A_{5I.Cyl.3m.x} = 1,40$		$A_{5I.Cyl.3m.\phi} = 1,15$
Short time:			$A_{5I.Cyl.sh.x} = 1,00$		$A_{5I.Cyl.sh.\phi} = 1,00$

Check for minimum design factors K and F:

If the value of K does not reach a minimum of 4 (advanced design) only for longtime loads, the  $A_{5B}$  values should be increased.

$$\text{Minimum } A_{5B} = \frac{K}{(A_1 \cdot A_2 \cdot A_3 \cdot A_4 \cdot \gamma_M \cdot \gamma_{F,i})} = \frac{4}{(1,0 \cdot 1,4 \cdot 1,20 \cdot 1,0 \cdot 1,4 \cdot 1,5)} = 1,13$$

If the value of F does not reach a minimum of 2,7 (advanced design) only for longtime loads, the  $A_{5I}$  values should be increased.

$$\text{Minimum } A_{5I} = \left( \frac{F}{A_1 \cdot A_2 \cdot A_3 \cdot A_4 \cdot \gamma_M \cdot \gamma_{F,i}} \right)^2 \geq 1,0 = \left( \frac{2,7}{1,0 \cdot 1,4 \cdot 1,20 \cdot 1,0 \cdot 1,4 \cdot 1,5} \right)^2 \geq 1,0 = 1,0$$

All  $A_5$  values are greater than the minimum  $A_5$  values.

### 10.2 Characteristic strength values

#### — For tension

$$\text{a) Axial } f_{Cyl.x.t.k} = 130 \text{ N/mm}^2 \quad \text{b) Tangential } f_{Cyl.\phi.t.k} = 400 \text{ N/mm}^2$$

#### — For bending

$$\text{a) Axial } f_{Cyl.x.b.k} = 150 \text{ N/mm}^2 \quad \text{b) Tangential } f_{Cyl.\phi.b.k} = 480 \text{ N/mm}^2$$

### 10.3 Moduli of elasticity

#### — For tension

$$\text{a) Axial } E_{Cyl.x.t} = 12500 \text{ N/mm}^2 \quad \text{b) Tangential } E_{Cyl.\phi.t} = 21000 \text{ N/mm}^2$$

#### — For bending

$$\text{a) Axial } E_{Cyl.x.b} = 12000 \text{ N/mm}^2 \quad \text{b) Tangential } E_{Cyl.\phi.b} = 19000 \text{ N/mm}^2$$

### 10.4 Analysis of the cylinder in axial direction

1. Step) Calculate all characteristic internal forces from the actions, which may cause internal forces in axial direction