



SLOVENSKI STANDARD

SIST-TS CEN/TS 1993-1-101:2023

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Evrokod 3: Projektiranje jeklenih konstrukcij - 1-101. del: Alternativna metoda interakcije za upogibne in tlačne elemente

Eurocode 3: Design of steel structures - Part 1-101: Alternative interaction method for members in bending and compression

Eurocode 3: Bemessung und Konstruktion von Stahlbauten - Teil 1 101: Alternative Interaktionsmethode für Bauteile unter Druck und Biegung

Eurocode 3: Calcul des structures en acier - Partie 1 101: Méthode de calcul pour la stabilité des barres en acier sollicitées en compression et flexion bi-axiale

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Ta slovenski standard je istoveten z: CEN/TS 1993-1-101:2022

ICS:

91.010.30	Tehnični vidiki	Technical aspects
91.080.13	Jeklene konstrukcije	Steel structures

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ICS 91.010.30; 91.080.13

English Version

**Eurocode 3: Design of steel structures - Part 1-101:
Alternative interaction method for members in bending
and compression**

Eurocode 3: Calcul des structures en acier - Partie 1
101: Méthode de calcul pour la stabilité des barres en
acier sollicitées en compression et flexion bi-axiale

Eurocode 3: Bemessung und Konstruktion von
Stahlbauten - Teil 1 101: Alternative
Interaktionsmethode für Bauteile unter Druck und
Biegung

This Technical Specification (CEN/TS) was approved by CEN on 22 August 2022 for provisional application.

The period of validity of this CEN/TS is limited initially to three years. After two years the members of CEN will be requested to submit their comments, particularly on the question whether the CEN/TS can be converted into a European Standard.

CEN members are required to announce the existence of this CEN/TS in the same way as for an EN and to make the CEN/TS available promptly at national level in an appropriate form. It is permissible to keep conflicting national standards in force (in parallel to the CEN/TS) until the final decision about the possible conversion of the CEN/TS into an EN is reached.

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EUROPEAN COMMITTEE FOR STANDARDIZATION
COMITÉ EUROPÉEN DE NORMALISATION
EUROPÄISCHES KOMITEE FÜR NORMUNG

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European foreword

This document (CEN/TS 1993-1-101:2022) has been prepared by Technical Committee CEN/TC 250 “Structural Eurocodes”, the secretariat of which is held by BSI. CEN/TC 250 is responsible for all Structural Eurocodes and has been assigned responsibility for structural and geotechnical design matters by CEN.

Attention is drawn to the possibility that some of the elements of this document may be the subject of patent rights. CEN shall not be held responsible for identifying any or all such patent rights.

This document has been prepared under Mandate M/515 issued to CEN by the European Commission and the European Free Trade Association.

This document has been drafted to be used in conjunction with relevant execution, material, product and test standards, and to identify requirements for execution, materials, products and testing that are relied upon by this document.

According to the CEN-CENELEC Internal Regulations, the national standards organizations of the following countries are bound to announce this Technical Specification: Austria, Belgium, Bulgaria, Croatia, Cyprus, Czech Republic, Denmark, Estonia, Finland, France, Germany, Greece, Hungary, Iceland, Ireland, Italy, Latvia, Lithuania, Luxembourg, Malta, Netherlands, Norway, Poland, Portugal, Republic of North Macedonia, Romania, Serbia, Slovakia, Slovenia, Spain, Sweden, Switzerland, Türkiye and the United Kingdom.

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CEN/TS 1993-1-101:2022 (E)**0 Introduction****0.1 Introduction to CEN/TS 1993-1-101**

EN 1993-1-1:2005 'Design of Steel Structures — General rules and rules for buildings' included two methods for the determination of interaction factors used for the verification of the buckling resistance of members under bending and axial compression force, which were provided in two separate Annexes A and B.

During the revision of EN 1993-1-1:2005, the standardization committee CEN/TC 250/SC 3 took a decision to retain the method provided in Annex B in EN 1993-1-1:2022 and to move the method of Annex A into a separate document, i.e. this document, for reasons of ease of use, without significant modifications.

This document is therefore to be considered as an alternative method to the one given in EN 1993-1-1:2022. Its applicability can be defined for each country, by the National Annex of EN 1993-1-1:2022.

This document, which was prepared in line with the Eurocodes, is intended for use by designers, clients, manufacturers, constructors, relevant authorities (in exercising their duties in accordance with national or international regulations), educators, software developers, and committees drafting standards for related product, testing and execution standards.

NOTE Some aspects of design are most appropriately specified by relevant authorities or, where not specified, can be agreed on a project-specific basis between relevant parties such as designers and clients. The Eurocodes identify such aspects making explicit reference to relevant authorities and relevant parties.

0.2 Verbal forms used in the Eurocodes

The verb "shall" expresses a requirement strictly to be followed and from which no deviation is permitted in order to comply with the Eurocodes.

The verb "should" expresses a highly recommended choice or course of action. Subject to national regulation and/or any relevant contractual provisions, alternative approaches could be used/adopted where technically justified.

The verb "may" expresses a course of action permissible within the limits of the Eurocodes.

The verb "can" expresses possibility and capability; it is used for statements of fact and clarification of concepts.

0.3 National annex for CEN/TS 1993-1-101

The applicability of this document can be defined for each country by the National Annex of EN 1993-1-1:2022.

The National Annex can contain, directly or by reference, non-contradictory complementary information for ease of implementation, provided it does not alter any provisions of the Eurocodes.

1 Scope

(1) This document provides an alternative method for the stability verification of steel members under compression axial force and bending moment, with reference to EN 1993-1-1.

NOTE For the applicability of this document, see Clause 4.

(2) The method given in this document applies to uniform steel members with double symmetric cross-section under axial compression force and bi-axial bending.

2 Normative references

The following documents are referred to in the text in such a way that some or all of their content constitutes requirements of this document. For dated references, only the edition cited applies. For undated references, the latest edition of the referenced document (including any amendments) applies.

EN 1993-1-1:2022, *Eurocode 3: Design of steel structures — Part 1-1: General rules and rules for buildings*

3 Terms, definitions and symbols

3.1 Terms and definitions

No terms and definitions are listed in this document.

3.2 Symbols

For the purposes of this document, the symbols given in EN 1993-1-1:2022 apply.

4 Use of this Technical Specification

(1) This document provides an alternative method for the stability verification of steel members under compression axial force and bending moment, with reference to EN 1993-1-1.

NOTE National choice on the application of this document is given in the National Annex of EN 1993-1-1:2022, see NDP regarding 8.3.3(2). If the National Annex contains no information on the application of this informative annex, it can be used.

5 Methodology

(1) The notation is the same as the one in EN 1993-1-1.

(2) Unless a second order analysis is carried out using the imperfections given in EN 1993-1-1:2022, 7.5, members which are subjected to combined bending and axial compression should satisfy the criteria given in Formulae (4.1) and (4.2):

$$\frac{N_{Ed}}{\chi_y N_{Rk}} + k_{yy} \frac{M_{y,Ed} + \Delta M_{y,Ed}}{\chi_{LT} M_{y,Rk}} + k_{yz} \frac{M_{z,Ed} + \Delta M_{z,Ed}}{M_{z,Rk}} \leq 1,0 \quad (4.1)$$

$$\frac{N_{Ed}}{\chi_z N_{Rk}} + k_{zy} \frac{M_{y,Ed} + \Delta M_{y,Ed}}{\chi_{LT} M_{y,Rk}} + k_{zz} \frac{M_{z,Ed} + \Delta M_{z,Ed}}{M_{z,Rk}} \leq 1,0 \quad (4.2)$$

where

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N_{Ed}	is the design value of the compression axial force;
N_{Rk}	is the characteristic value of the resistance to the axial force according to EN 1993-1-1:2022, 8.2.2.6;
$M_{y,Ed}$ and $M_{z,Ed}$	are the design values of the maximum bending moments about the y-y and z-z axes respectively, along the member;
$\Delta M_{y,Ed}$, $\Delta M_{z,Ed}$	are the moments due to the shift of the centroidal axis according to EN 1993-1-1:2022, 8.2.2.5 for Class 4 sections;
$M_{y,Rk}$ and $M_{z,Rk}$	are the characteristic values of the resistance to the bending moment about the y-y axis and z-z axis respectively, according to EN 1993-1-1:2022, 8.2.2.6;
χ_y and χ_z	are the reduction factors due to flexural buckling according to EN 1993-1-1:2022, 8.3.1;
χ_{LT}	is the reduction factor due to lateral torsional buckling according to EN 1993-1-1:2022, 8.3.2;
k_{yy} , k_{yz} , k_{zy} , k_{zz}	are the interaction factors according to Table 4.1.

(3) As a simplification for uniform members subjected to axial compression and bending about the strong axis, the condition given in Formula (4.3) may be used when out-of-plane buckling (flexural buckling and lateral torsional buckling) is prevented:

$$\frac{1,1 N_{Ed}}{\chi_y N_{Rk}} + C_{my,0} \frac{1,1 (M_{y,Ed} + \Delta M_{y,Ed})}{M_{y,Rk}} \leq 1,0 \quad (4.3)$$

where

γ_{M1}	is the partial factor according to the appropriate part of EN 1993: Part 1-1 for buildings; Part 2 for bridges.
$C_{my,0}$	is the equivalent uniform moment factor according to Table 4.2.

Table 4.1 — Interaction factors k_{ij}

Interaction factors	Design assumptions	
	Elastic cross-sectional properties For Class 3 with $W_i = W_{el,i}$ and Class 4	Plastic cross-sectional properties for Class 1 and Class 2 ($W_i = W_{pl,i}$) or Elasto-plastic cross-sectional properties for Class 3 (with $W_i = W_{ep,i}$ according to EN 1993-1-1:2022, Annex B)
k_{yy}	$C_{my} C_{mLT} \frac{\mu_y}{1 - \frac{N_{Ed}}{N_{cr,y}}}$	$C_{my} C_{mLT} \frac{\mu_y}{1 - \frac{N_{Ed}}{N_{cr,y}}} \frac{1}{C_{yy}}$
k_{yz}	$C_{mz} \frac{\mu_y}{1 - \frac{N_{Ed}}{N_{cr,z}}}$	$C_{mz} \frac{\mu_y}{1 - \frac{N_{Ed}}{N_{cr,z}}} \frac{1}{C_{yz}} 0,6 \sqrt{\frac{w_z}{w_y}}$
k_{zy}	$C_{my} C_{mLT} \frac{\mu_z}{1 - \frac{N_{Ed}}{N_{cr,y}}}$	$C_{my} C_{mLT} \frac{\mu_z}{1 - \frac{N_{Ed}}{N_{cr,y}}} \frac{1}{C_{zy}} 0,6 \sqrt{\frac{w_y}{w_z}}$
k_{zz}	$C_{mz} \frac{\mu_z}{1 - \frac{N_{Ed}}{N_{cr,z}}}$	$C_{mz} \frac{\mu_z}{1 - \frac{N_{Ed}}{N_{cr,z}}} \frac{1}{C_{zz}}$
Auxiliary values:		
$\mu_y = \frac{1 - \frac{N_{Ed}}{N_{cr,y}}}{1 - \chi_y \frac{N_{Ed}}{N_{cr,y}}}$ $\mu_z = \frac{1 - \frac{N_{Ed}}{N_{cr,z}}}{1 - \chi_z \frac{N_{Ed}}{N_{cr,z}}}$ $w_y = \frac{W_y}{W_{el,y}} \leq 1,5$ $w_z = \frac{W_z}{W_{el,z}} \leq 1,5$ $n_{pl} = \frac{N_{Ed}}{N_{Rk} / \gamma_{M0}}$ $a_{LT} = 1 - \frac{I_T}{I_y} \geq 0$	$C_{yy} = 1 + (w_y - 1) \left[\left(2 - \frac{1,6}{w_y} C_{my}^2 \bar{\lambda}_{max}^{-2} - \frac{1,6}{w_y} C_{my}^2 \bar{\lambda}_{max}^{-2} \right) n_{pl} - b_{LT} \right] \geq \frac{W_{el,y}}{W_y}$ <p>with: $b_{LT} = 0,5 a_{LT} \frac{\bar{\lambda}_0^{-2}}{\chi_{LT} M_{y,Rk} / \gamma_{M1}} \frac{M_{y,Ed}}{M_{z,Rk} / \gamma_{M1}}$</p> $C_{yz} = 1 + (w_z - 1) \left[\left(2 - 14 \frac{C_{mz}^2 \bar{\lambda}_{max}^{-2}}{w_z^5} \right) n_{pl} - c_{LT} \right] \geq 0,6 \sqrt{\frac{w_z}{w_y}} \frac{W_{el,z}}{W_z}$ <p>with: $c_{LT} = 10 a_{LT} \frac{\bar{\lambda}_0^{-2}}{5 + \bar{\lambda}_z^{-4}} \frac{M_{y,Ed}}{C_{my} \chi_{LT} M_{y,Rk} / \gamma_{M1}}$</p> $C_{zy} = 1 + (w_y - 1) \left[\left(2 - 14 \frac{C_{my}^2 \bar{\lambda}_{max}^{-2}}{w_y^5} \right) n_{pl} - d_{LT} \right] \geq 0,6 \sqrt{\frac{w_y}{w_z}} \frac{W_{el,y}}{W_y}$ <p>with: $d_{LT} = 2 a_{LT} \frac{\bar{\lambda}_0}{0,1 + \bar{\lambda}_z^{-4}} \frac{M_{y,Ed}}{C_{my} \chi_{LT} M_{y,Rk} / \gamma_{M1}} \frac{M_{z,Ed}}{C_{mz} M_{z,Rk} / \gamma_{M1}}$</p> $C_{zz} = 1 + (w_z - 1) \left[2 - \frac{1,6}{w_z} C_{mz}^2 \bar{\lambda}_{max} (1 + \bar{\lambda}_{max}) - e_{LT} \right] n_{pl} \geq \frac{W_{el,z}}{W_z}$ <p>with: $e_{LT} = 1,7 a_{LT} \frac{\bar{\lambda}_0}{0,1 + \bar{\lambda}_z^{-4}} \frac{M_{y,Ed}}{C_{my} \chi_{LT} M_{y,Rk} / \gamma_{M1}}$</p> <p>where $M_{y,Rk}$ and $M_{z,Rk}$ are defined in EN 1993-1-1:2022, 8.2.2.6; W_y and W_z are defined in EN 1993-1-1:2022, Table 8.1.</p>	

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Interaction factors	Design assumptions	
	Elastic cross-sectional properties For Class 3 with $W_i = W_{el,i}$ and Class 4	Plastic cross-sectional properties for Class 1 and Class 2 ($W_i = W_{pl,i}$) or Elasto-plastic cross-sectional properties for Class 3 (with $W_i = W_{ep,i}$ according to EN 1993-1-1:2022, Annex B)
$\bar{\lambda}_{\max} = \max \left\{ \begin{array}{l} \bar{\lambda}_y \\ \bar{\lambda}_z \end{array} \right.$ $\bar{\lambda}_0 = \text{relative slenderness for lateral-torsional buckling due to uniform bending moment, i.e. } \psi_y = 1,0 \text{ in Table 4.2;}$ $\bar{\lambda}_{LT} = \text{relative slenderness for lateral-torsional buckling.}$		
<p>If: $\bar{\lambda}_0 \leq 0,2\sqrt{C_1} \sqrt[4]{\left(1 - \frac{N_{Ed}}{N_{cr,z}}\right)\left(1 - \frac{N_{Ed}}{N_{cr,T}}\right)}$</p>	$C_{my} = C_{my,0}$ $C_{mz} = C_{mz,0}$ $C_{mLT} = 1,0$	
<p>If $\bar{\lambda}_0 > 0,2\sqrt{C_1} \sqrt[4]{\left(1 - \frac{N_{Ed}}{N_{cr,z}}\right)\left(1 - \frac{N_{Ed}}{N_{cr,T}}\right)}$</p>	$C_{my} = C_{my,0} + (1 - C_{my,0}) \frac{\sqrt{\varepsilon_y} a_{LT}}{1 + \sqrt{\varepsilon_y} a_{LT}}$ $C_{mz} = C_{mz,0}$ $C_{mLT} = C_{my}^2 \frac{a_{LT}}{\sqrt{\left(1 - \frac{N_{Ed}}{N_{cr,z}}\right)\left(1 - \frac{N_{Ed}}{N_{cr,T}}\right)}} \geq 1,0$	
<p>C_1</p> <p>factor depending on the loading and end conditions. It may be taken as:</p> $C_1 = k_c^{-2}$ <p>where</p> <p>k_c is to be taken from EN 1993-1-1:2022, Table 8.6.</p>		
<p>$C_{mi,0}$</p> <p>equivalent uniform moment factors to be determined according to Table 4.2</p>		
$\varepsilon_y = \frac{M_{y,Ed}}{N_{Ed}} \frac{A}{W_{el,y}}$ <p>for Class 1, 2 and 3 cross-sections</p>		
$\varepsilon_y = \frac{M_{y,Ed}}{N_{Ed}} \frac{A_{eff}}{W_{eff,y}}$ <p>for Class 4 cross-sections</p>		
<p>$N_{cr,y}$</p> <p>elastic critical axial force for flexural buckling about y-y axis</p>		
<p>$N_{cr,z}$</p> <p>elastic critical axial force for flexural buckling about z-z axis</p>		
<p>$N_{cr,T}$</p> <p>elastic critical axial force for torsional buckling</p>		
<p>I_T</p> <p>St Venant torsional constant</p>		
<p>I_y</p> <p>second moment of area about the y-y axis</p>		