



SLOVENSKI STANDARD SIST EN 384:2016+A1:2019

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Nadomešča:
SIST EN 384:2016

Konstruktivski les - Ugotavljanje karakterističnih vrednosti mehanskih lastnosti in gostote

Structural timber - Determination of characteristic values of mechanical properties and density

Bauholz für tragende Zwecke - Bestimmung charakteristischer Werte für mechanische Eigenschaften und Rohdichte

Bois de structure - Détermination des valeurs caractéristiques des propriétés mécaniques et de la masse volumique

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91.080.20	Lesene konstrukcije	Timber structures

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EUROPEAN STANDARD

EN 384:2016+A1

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English Version

Structural timber - Determination of characteristic values of mechanical properties and density

Bois de structure - Détermination des valeurs
caractéristiques des propriétés mécaniques et de la
masse volumique

Bauholz für tragende Zwecke - Bestimmung
charakteristischer Werte für mechanische
Eigenschaften und Rohdichte

This European Standard was approved by CEN on 30 January 2016 and includes Amendment 1 approved by CEN on 8 October 2018.

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COMITÉ EUROPÉEN DE NORMALISATION
EUROPÄISCHES KOMITEE FÜR NORMUNG

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European foreword

This document (EN 384:2016+A1:2018) has been prepared by Technical Committee CEN/TC 124 “Timber structures”, the secretariat of which is held by AFNOR.

This European Standard shall be given the status of a national standard, either by publication of an identical text or by endorsement, at the latest by May 2019, and conflicting national standards shall be withdrawn at the latest by May 2019.

Attention is drawn to the possibility that some of the elements of this document may be the subject of patent rights. CEN shall not be held responsible for identifying any or all such patent rights.

This document includes Amendment 1 approved by CEN on 2018-10-08.

This document supersedes A1 EN 384:2016 A1.

The start and finish of text introduced or altered by amendment is indicated in the text by tags A1 A1.

A1 Deleted text A1

According to the CEN/CENELEC Internal Regulations, the national standards organizations of the following countries are bound to implement this European Standard: Austria, Belgium, Bulgaria, Croatia, Cyprus, Czech Republic, Denmark, Estonia, Finland, Former Yugoslav Republic of Macedonia, France, Germany, Greece, Hungary, Iceland, Ireland, Italy, Latvia, Lithuania, Luxembourg, Malta, Netherlands, Norway, Poland, Portugal, Romania, Serbia, Slovakia, Slovenia, Spain, Sweden, Switzerland, Turkey and the United Kingdom.

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Introduction

Structural design codes can only function effectively if standard methods of determining the mechanical and physical properties exist. The aim of the procedures given in this standard is to derive characteristic values that are comparable in terms of the populations they represent. The standard permits the use of as much existing test data as possible from various sampling and testing techniques.

Where methods are given to permit characteristic values to be determined from a less than ideal amount of structural size test data, reduction factors to reflect a lower degree of confidence are employed.

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1 Scope

This European Standard gives a method for determining characteristic values of mechanical properties and density, for defined populations of visual grades and/or strength classes of machine graded structural timber. Additionally it covers the stages of sampling, testing, analysis and presentation of the data.

The standard provides methods to derive strength, stiffness and density properties for structural timber from tests with defect-free specimen.

The values determined in accordance with this standard for mechanical properties and density are suitable for assigning grades and species to the strength classes of EN 338.

NOTE 1 For assigning grades and species to the strength classes in EN 338 only three properties, i.e. bending or tension strength, modulus of elasticity parallel to grain in bending or tension and density need to be determined from test data, other properties can be calculated according to Table 2.

NOTE 2 EN 1912 gives examples of established visual grades assigned to strength classes.

2 Normative references

The following documents, in whole or in part, are normatively referenced in this document and are indispensable for its application. For dated references, only the edition cited applies. For undated references, the latest edition of the referenced document (including any amendments) applies.

EN 338, *Structural timber — Strength classes*

EN 408, *Timber structures — Structural timber and glued laminated timber — Determination of some physical and mechanical properties*

EN 13183-2, *Moisture content of a piece of sawn timber — Part 2: Estimation by electrical resistance method*

EN 13183-3, *Moisture content of a piece of sawn timber — Part 3: Estimation by capacitance method*

EN 14081-1:2016, *Timber structures — Strength graded structural timber with rectangular cross section — Part 1: General requirements*

EN 14081-2, *Timber structures — Strength graded structural timber with rectangular cross section — Part 2: Machine grading; additional requirements for initial type testing*

EN 14081-3, *Timber structures — Strength graded structural timber with rectangular cross section — Part 3: Machine grading; additional requirements for factory production control*

EN 14358:2016, *Timber structures — Calculation and verification of characteristic values*

3 Terms and definitions

For the purposes of this document, the following terms and definitions apply.

3.1

characteristic value

representative value of a material property used for design, which is based either on 5-percentile values (e.g. strength properties and density) or mean values (e.g. modulus of elasticity)

EN 384:2016+A1:2018 (E)**3.2*****p*-percentile**

value for which the probability of getting lower values is p %

3.3**population**

timber for which the characteristic values are relevant

3.4**timber source**

identifiable geographical origin of a species or species combination from which timber is, or is intended to be, strength graded

3.5**sample**

a number of ungraded specimens of one timber species or species combination, one source, with sizes and quality representative of the timber population (see 5.1)

3.6**sub-sample**

part of one or more samples consisting of specimens of one grade

3.7**small clear test**

test to determine mechanical properties of small defect-free specimens

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3.8**specimen**

piece of timber from which the test piece is taken

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3.9**thickness**

lesser dimension perpendicular to the longitudinal axis of a piece of timber

3.10**width**

larger dimension perpendicular to the longitudinal axis of a piece of timber

3.11**depth**

in the case of bending, cross-sectional dimension parallel to the direction of loading; in the case of tension, the width

4 Symbols and abbreviations

a_f	distance between the inner load points of the bending test (in mm)
E_0	modulus of elasticity parallel to grain (in N/mm ²)
$E_{0,mean}$	mean characteristic value of modulus of elasticity parallel to grain (in N/mm ²)
$E_{0,k}$	5-percentile characteristic value of modulus of elasticity parallel to grain (in N/mm ²)
$E_{90,mean}$	mean characteristic value of modulus of elasticity perpendicular to grain (in N/mm ²)
\bar{E}_i	mean modulus of elasticity for one sub-sample (in N/mm ²)

$\bar{E}_{i,\min}$	lowest mean modulus of elasticity of all sub-samples (in N/mm ²)
$E_{m,\text{global}}$	global modulus of elasticity in bending (in N/mm ²)
$E_{m,\text{local}}$	local modulus of elasticity in bending (in N/mm ²)
f	strength property
$f_{c,0,k}$	5- percentile characteristic value of compression strength parallel to grain (in N/mm ²)
$f_{c,90,k}$	5- percentile characteristic value of compression strength perpendicular to grain (in N/mm ²)
f_k	5- percentile characteristic value of strength (in N/mm ²)
$f_{m,k}$	5- percentile characteristic value of bending strength (in N/mm ²)
$f_{t,0,k}$	5- percentile characteristic value of tension strength parallel to grain (in N/mm ²)
$f_{t,90,k}$	5- percentile characteristic value of tension strength perpendicular to grain (in N/mm ²)
$f_{05,i}$	5-percentile value for each sub-sample (in N/mm ²)
$f_{05,i,\min}$	lowest 5-percentile value of all sub-samples (in N/mm ²)
$f_{v,k}$	5- percentile characteristic value of shear strength (in N/mm ²)
G_{mean}	mean characteristic value of shear modulus (in N/mm ²)
h	depth (in mm)
k_h	factor for adjusting f when h is not 150 mm
k_l	factor for adjusting f when test span is not 18 h
k_n	factor to adjust for the number of sub-samples
k_v	factor to allow for the lower variability of f_{05} values between sub-samples for machine grades in comparison with visual grades
ℓ	span (in mm)
ℓ_{et}	effective length for the test (in mm)
n	total number of specimens
n_i	number of specimens in a sub-sample
n_s	number of sub-samples
u	moisture content (in %)
u_{ref}	reference moisture content, normally at 12 %
ρ	density (in kg/m ³)
ρ_{mean}	mean density (in kg/m ³)
ρ_k	characteristic density (5-percentile) (in kg/m ³)
$\rho_{05,i}$	5-percentile density for a sub-sample (in kg/m ³)
$\rho_{05,i,\min}$	lowest 5-percentile density of all sub-samples (in kg/m ³)

5 Mechanical properties determined from full-size specimens

5.1 Sampling

The sampling shall be representative of the population.

Any known or suspected difference in the mechanical properties of the population due to e.g. sawmills, tree size, countries or silviculture shall be represented within the sampling by a similar proportion to their frequency in the population. This shall be the major influence in determining the number and size of samples.

Samples shall be selected from one source of timber and shall be graded visually or by machine to sub-samples according to the requirements given in EN 14081-1.

For visual grading, each sub-sample shall consist of at least 40 specimens and be of one source.

For bending and tension parallel to grain tests, specimens shall have a sufficient length so that critical defects can be located in the critical test zone (see 5.2). A length of at least 30 times the depth or 3.6 m whichever is the lesser meets this requirement.

For the determination of strengths perpendicular to the grain and shear strength clear specimens shall be sampled.

5.2 Testing

Testing shall be carried out in accordance with EN 408 for strength, modulus of elasticity, density and moisture content. For bending parallel to grain, tension parallel to grain or modulus of elasticity, a critical section shall be selected in each piece of timber. This section is the position at which failure is expected to occur and therefore determines the grade for that piece. For bending the tension edge shall be selected at random. Whenever possible the critical section shall be placed inside the inner load points in a bending test or between the jaws in a tension test (centrally if possible). If this is not possible, the second most critical section shall be tested and determines the grade for that piece.

Existing historical data (before 1995) from different test methods or moisture conditions are acceptable provided sufficient information exists to adjust the results to the reference conditions given in 5.3.

5.3 Reference conditions

5.3.1 Moisture content

The reference moisture content shall be consistent with a temperature of 20 °C and 65 % relative humidity.

NOTE For most timber species this corresponds to a moisture content of about 12 %.

For specimens not tested to failure, the moisture content of each specimen is permitted to be determined from EN 13183-2 or EN 13183-3.

5.3.2 Bending strength

The reference condition corresponds to bending to a depth of 150 mm and to the standard test set-up proportions of third point loading with an overall span of 18 times the specimen depth.

5.3.3 Tension strength

The reference condition corresponds to a depth of 150 mm.

5.3.4 Density

Density is determined on small defect-free prisms according EN 408.

For specimens not tested to failure, the density of each specimen is permitted to be determined from the mass and volume of the test piece and adjusted to the density of the small defect-free prisms, by dividing by 1,05 in case of softwood. For hardwood no adjustment is necessary.

Adjustment for moisture content may also be necessary.

5.4 Adjustment factors

5.4.1 General

Test results shall be adjusted, piece by piece, to the standard reference conditions as given in 5.3.

If historical data (before 1995) is being used and records for individual specimen are incomplete, sub-sample 5-percentile or mean value shall be adjusted.

5.4.2 Moisture content

Test values for compression parallel to the grain, modulus of elasticity parallel to the grain and density of specimens not tested at the reference moisture content shall be adjusted either:

- by adjustment factors derived from tests;
- or by Formulae (1), (2) or (3).

$$f_{c,0} = f_{c,0}(u)(1 + 0,03(u - u_{\text{ref}})) \quad (1)$$

$$E_0 = E_0(u)(1 + 0,01(u - u_{\text{ref}})) \quad (2)$$

$$\rho = \rho(u)(1 - 0,005(u - u_{\text{ref}})) \quad (3)$$

where

$f_{c,0}$ is the compression strength parallel to the grain;

E_0 is the modulus of elasticity parallel to the grain;

ρ is the density;

u is the moisture content at testing ($8 \% \leq u \leq 18 \%$)

u_{ref} is the reference moisture content, normally $u_{\text{ref}} = 12 \%$ (see 5.3.1).

For the adjustment of compression strength parallel to the grain and the modulus of elasticity u shall be taken as 18 % for moisture contents higher than 18 %.

If the moisture content u is lower than 8 %, special consideration is required for the adjustment of strength properties, modulus of elasticity and density.

For the adjustment of density special consideration is required for moisture contents above fibre saturation.

If other more relevant factors are available from test data, then they shall be used instead.

5.4.3 Timber size and test length

For depth less than 150 mm, and characteristic density less than or equal to 700 kg/m^3 , bending and tension strength shall be adjusted to 150 mm depth by dividing by the factor k_h from Formula (4):

$$k_h = \text{Min} \left\{ \begin{array}{l} \left(\frac{150}{h} \right)^{0,2} \\ 1,3 \end{array} \right. \quad (4)$$