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Standard Specification for Structural Steel for Bridges¹

This standard is issued under the fixed designation A709/A709M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

1. Scope*

1.1 This specification covers carbon and high-strength low-alloy steel structural shapes, plates, and bars and quenched and tempered alloy steel for structural plates intended for use in bridges. Seven grades are available in four yield strength levels as follows:

Grade U.S. [SI]	Yield Strength, ksi [MPa]
36 [250]	36 [250]
50 [345]	50 [345]
50S [345S]	50 [345]
50W [345W]	50 [345]
HPS 50W [HPS 345W]	50 [345]
HPS 70W [HPS 485W]	70 [485]
HPS 100W [HPS 690W]	100 [690]

1.1.1 Grades 36 [250], 50 [345], 50S [345S], and 50W [345W] are also included in Specifications A36/A36M, A572/A572M, A992/A992M, and A588/A588M, respectively. When the supplementary requirements of this specification are specified, they exceed the requirements of Specifications A36/A36M, A572/A572M, A992/A992M, and A588/A588M.

1.1.2 Grades 50W [345W], HPS 50W [HPS 345W], HPS 70W [HPS 485W], and HPS 100W [HPS 690W] have enhanced atmospheric corrosion resistance (see 13.1.2). Product availability is shown in Table 1.

1.2 Grade HPS 70W [HPS 485W] or HPS 100W [HPS 690W] shall not be substituted for Grades 36 [250], 50 [345], 50S [345S], 50W [345W], or HPS 50W [HPS 345W]. Grade 50W [345W], or HPS 50W [HPS 345W] shall not be substituted for Grades 36 [250], 50 [345] or 50S [345S] without agreement between the purchaser and the supplier.

1.3 When the steel is to be welded, it is presupposed that a welding procedure suitable for the grade of steel and intended use or service will be utilized. See Appendix X3 of Specification A6/A6M for information on weldability.

1.4 For structural products to be used as tension components requiring notch toughness testing, standardized requirements are provided in this standard, and they are based upon American Association of State Highway and Transportation Officials (AASHTO) requirements for both fracture critical and non-fracture critical members.

1.5 Supplementary requirements are available but shall apply only if specified in the purchase order.

1.6 The values stated in either SI units or inch-pound units are to be regarded separately as standard. The values stated in each system may not be exact equivalents; therefore, each system shall be used independently of the other. Combining values from the two systems may result in non-conformance with the standard.

1.7 For structural products produced from coil and furnished without heat treatment or with stress relieving only, the additional requirements, including additional testing requirements and the reporting of additional test results, of Specification A6/A6M apply.

2. Referenced Documents

2.1 ASTM Standards:²

A6/A6M Specification for General Requirements for Rolled Structural Steel Bars, Plates, Shapes, and Sheet Piling A36/A36M Specification for Carbon Structural Steel

A370 Test Methods and Definitions for Mechanical Testing of Steel Products

A572/A572M Specification for High-Strength Low-Alloy Columbium-Vanadium Structural Steel

A588/A588M Specification for High-Strength Low-Alloy Structural Steel, up to 50 ksi [345 MPa] Minimum Yield Point, with Atmospheric Corrosion Resistance

*A Summary of Changes section appears at the end of this standard.

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² For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.



TABLE 1 Tensile and Hardness Requirements^A

NOTE 1—Where "..." appears in this table, there is no requirement.

			Yield Point			Minimum El	ongation, %		
	Plate Thickness.	Structural Shape Flange	or Yield	Tensile	Plates an	d Bars ^{C,E}	Sha	oes ^E	Reduc- tion of
Grade	in. [mm]	or Leg Thickness, in. [mm]	Strength, ^{<i>B</i>} ksi [MPa]	Strength, ksi [MPa]	8 in. or 200 mm	2 in. or 50 mm	8 in. or 200 mm	2 in. or 50 mm	Area ^{<i>C,D</i>} min, %
36 [250] <u>36 [250]</u> -	to 4 [100], incl <u>to 4 [100], incl</u> -	to 3 in. [75 mm], incl to 3 in. [75 mm], incl over 3 in. [75 mm] over 3 in. [75 mm]	<u>36 [250] min</u> 36 [250] min 36 [250] min 36 [250] min	58 80 [400 550] 58-80 [400-550] 58 [400] min 58 [400] min	<u>-20</u> 20 		20 -20 20 20	21 21 19 19	
50 [345] 50 [345] 50S [345S]	- to 4 [100], incl to 4 [100], incl <u></u>	- all <u>all</u> - all	50 [345] min 50 [345] min 50 65 [345 450] ^{HI}	65 [450] min 65 [450] min 65 [450]^H min	18 18 	<u></u>	20 18 18 18	<u>19</u> 21^F 21 ^F 21	
<u>50S [345S]</u>	G 	all	<u>50–65</u> [345–450] ^{<i>HI</i>}	65 [450] ^H min	<u> </u>		18	21	<u> </u>
50W [345W] and — HPS 50W — [HPS 345W]	-to-4 [100], incl	all	<u>50 [345] min</u>	-70 [485] min		_21	—18	21^J	
50W [345W] and HPS 50W [HPS 345W]	<u>to 4 [100], incl</u>	all	50 [345] min	70 [485] min	<u>18</u>	21	18	21 ^{_J}	<u></u>
H PS 70W 	-to-4 [100], incl	G	— 70 [485] min^B	-85-110 [585-760]		—19 ^K			
HPS 70W [HPS 485 W]	<u>to 4 [100], incl</u>	G -	70 [485] min ^B	85-110 [585-760]		<u>19^K</u>	<u> </u>		<u> </u>
HPS 100W 	-to-2½ [65], incl	<u> </u>	-100 [690] min ^B	-110-130 [760-895]	S —	—18 ^K	- <u></u>		<u>_</u>
HPS 100W [HPS 690W]	to 21/2 [65], incl	(h i ttos:	<u>100 [690] min^B</u>	<u>110–130 [760–895]</u>	ten.	18 ^K		<u></u>	
	over 21/2 to 4 [65 to 100], incl ^M		90 [620] min ^{<i>B</i>}	100–130 [690–895]		16 ^K			L

^A See specimen orientation and preparation subsection in the Tension Tests section of Specification A6/A6M.

^B Measured at 0.2 % offset or 0.5 % extension under load as described in Section 13 of Test Methods A370.

^C Elongation and reduction of area not required to be determined for floor plates.

^D For plates wider than 24 in. [600 mm], the reduction of area requirement, where applicable, is reduced by five percentage points.

^E For plates wider than 24 in. [600 mm], the elongation requirement is reduced by two percentage points. See elongation requirement adjustments in the Tension Tests section of Specification A6/A6M.

^F Elongation in 2 in. or 50 mm: 19 % for shapes with flange thickness over 3 in. [75 mm]. J-4CIC-ad10-b0/94 [9661a/astm-a/09-a/09m-09a] ^G Not applicable.

^H The yield to tensile ratio shall be 0.87 or less for shapes that are tested from the web location; for all other shapes, the requirement is 0.85.

A maximum yield strength of 70 ksi [480 MPa] is permitted for structural shapes that are required to be tested from the web location.

^J For wide flange shapes with flange thickness over 3 in. [75 mm], elongation in 2 in. or 50 mm of 18 % minimum applies.

^K If measured on the Fig. 3 (Test Methods A370) 1½-in. [40-mm] wide specimen, the elongation is determined in a 2-in. or 50-mm gage length that includes the fracture and shows the greatest elongation.

^L 40 % minimum applies if measured on the Fig 3 (Test Methods A370) 1½-in. [40-mm] wide specimen; 50 % minimum applies if measured on the Fig. 4 (Test Methods A370) ½-in. [12.5-mm] round specimen.

^M Not applicable to Fracture Critical Tension Components (see Table 9).

TABLE 2 Grade 36 [250] Chemical Requirements (Heat Analysis)

NOTE 1—Where "..." appears in this table there is no requirement. The heat analysis for manganese shall be determined and reported as described in the Heat Analysis section of Specification A6/A6M.

Product		Plates ^B			Bars ^B			
Thickness, in. (mm)	Shapes ^A All	To ¾ [20], incl	Over ³ ⁄ ₄ to 1 ¹ ⁄ ₂ [20 to 40], incl	Over 1½ to 2½ [40 to 65], incl	Over 2½ to 4 [65 to 100], incl	To ¾ [20], incl	Over ¾ to 1½ [20 to 40], incl	Over 1½ to 4 [100], incl
Carbon, max, %	0.26	0.25	0.25	0.26	0.27	0.26	0.27	0.28
Manganese, %			0.80-1.20	0.80-1.20	0.85-1.20		0.60-0.90	0.60-0.90
Phosphorus, max, %	0.04	0.04	0.04	0.04	0.04	0.04	0.04	0.04
Sulfur, max, %	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05
Silicon, %	0.40 max	0.40 max	0.40 max	0.15-0.40	0.15-0.40	0.40 max	0.40 max	0.40 max
Copper, min, % when copper steel is specifie	0.20 ed	0.20	0.20	0.20	0.20	0.20	0.20	0.20

^A Manganese content of 0.85 to 1.35 % and silicon content of 0.15 to 0.40 % is required for shapes with flange thickness over 3 in. [75 mm].

^B For each reduction of 0.01 % below the specified carbon maximum, an increase of 0.06 % manganese above the specified maximum will be permitted up to a maximum of 1.35 %.

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TABLE 3 Grade 50 [345] Chemical Requirements^A (Heat Analysis)

						Silicon ^C	Columbium, Vanadium and Nitrogen
Maximum Diameter, Thickness, or Distance Between Parallel Faces, in. [mm]	Carbon, max, %	Manganese, ^{<i>B</i>} max, %	Phosphorus, max, %	Sulfur, max, %	Plates to 1½-in. [40-mm] Thick, Shapes with flange or leg thickness to 3 in. [75 mm] inclusive, Sheet Piling, Bars, Zees, and Rolled Tees, max, % ^D	Plates Over 1½-in. [40-mm] Thick and Shapes with flange thickness over 3 in. [75 mm], %	
4 [100]	0.23	1.35	0.04	0.05	0.40	0.15-0.40	E

^A Copper when specified shall have a minimum content of 0.20 % by heat analysis (0.18 % by product analysis).

^B Manganese, minimum by heat analysis of 0.80 % (0.75 % by product analysis) shall be required for all plates over % in. [10 mm] in thickness; a minimum of 0.50 % (0.45 % by product analysis) shall be required for plates % in. [10 mm] and less in thickness, and for all other products. The manganese to carbon ratio shall not be less than 2 to 1. For each reduction of 0.01 percentage point below the specified carbon maximum, an increase of 0.06 percentage point manganese above the specified maximum is permitted, up to a maximum of 1.60 %.

^C Silicon content in excess of 0.40 % by heat analysis must be negotiated.

^D Bars over 1½ in. [40 mm] in diameter, thickness, or distance between parallel faces, shall be made by a killed steel practice.

^E Alloy content shall be in accordance with Type 1, 2, 3, or 5 and the contents of the applicable elements shall be reported on the test report.

Туре	Elements	Heat Analysis, %
1	Columbium ^A	0.005–0.05 ^B
2	Vanadium	0.01-0.15
3	Columbium ^A	0.005–0.05 ^B
	Vanadium	0.01-0.15
	Columbium plus vanadium	0.02–0.15 ^C
5	Titanium	0.006-0.04
	Nitrogen	0.003-0.015
	Vanadium	0.06 max

^A Columbium shall be restricted to Grade 50 [345] plate, bar, zee, and rolled tee thickness of ³/₄ in. [20 mm] max, and to shapes with flange or leg thickness to 1¹/₂ in. [40 mm] inclusive unless killed steel is furnished. Killed steel shall be confirmed by a statement of killed steel on the test report, or by a report of the presence of a sufficient quantity of a strong deoxidizing element, such as silicon at 0.10 % or higher, or aluminum at 0.015 % or higher.

^{*B*} Product analysis limits = 0.004 to 0.06 %. ^{*C*} Product analysis limits = 0.01 to 0.16 %.

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s//standards.iteh.al/catalog/standards/sist/355cccc3e-cd00-4ctc-ad16-b0//9419e61a/astm-a/09-a/09m-09a TABLE 4 Grade 50W [345 W] Chemical Requirements (Heat Analysis)

NOTE 1—Types A, B, and C are equivalent to Specification A588/ A588M Grades A, B, and C, respectively.

Element		Composition, % ^A				
	Type A	Type B	Type C			
Carbon ^B	0.19 max	0.20 max	0.15 max			
Manganese ^B	0.80-1.25	0.75-1.35	0.80-1.35			
Phosphorus	0.04 max	0.04 max	0.04 max			
Sulfur	0.05 max	0.05 max	0.05 max			
Silicon	0.30-0.65	0.15-0.50	0.15-0.40			
Nickel	0.40 max	0.50 max	0.25-0.50			
Chromium	0.40-0.65	0.40-0.70	0.30-0.50			
Copper	0.25-0.40	0.20-0.40	0.20-0.50			
Vanadium	0.02-0.10	0.01-0.10	0.01-0.10			

^A Weldability data for these types have been qualified by FHWA for use in bridge construction.

^{*B*} For each reduction of 0.01 percentage point below the specified maximum for carbon, an increase of 0.06 percentage point above the specified maximum for manganese is permitted, up to a maximum of 1.50 %.

A673/A673M Specification for Sampling Procedure for Impact Testing of Structural Steel A992/A992M Specification for Structural Steel Shapes

G101 Guide for Estimating the Atmospheric Corrosion Resistance of Low-Alloy Steels

3. Terminology

3.1 Definitions of Terms Specific to This Standard:



TABLE 5 Grades HPS 50W [HPS 345W] and HPS 70W [HPS 485 W], and HPS 100W [HPS 690W] Chemical Requirements (Heat Analysis)

Note 1—	Where "" appears in t	his table, there is no re	equirement.
	Composit	ion, %	
Element—	Grades HPS 50W [HPS 345W], HPS 70W [HPS 485W]	Grade HPS 100W [HPS 690W]	
	Carbon Manganese	0.11 max	0.08 max
	2.5 in. [65 mm] and under	1.10-1.35	0.95-1.50
	- Over 2.5 in. [65 mm]	1.10-1.50	A
	Over 2.5 in. [65 mm]	1.10-1.50	0.95-1.50
	Phosphorus	0.020 max	0.015 max
	Sulfur ^B	0.006 max	0.006 max
	<u>Sulfur</u> ^A	0.006 max	0.006 max
	Silicon	0.30-0.50	0.15–0.35
	Copper	0.25-0.40	0.90-1.20
	Nickel	0.25-0.40	0.65–0.90
	Chromium	0.45-0.70	0.40-0.65
	Molybdenum	0.02-0.08	0.40-0.65
	Vanadium	0.04–0.08	0.04-0.08
	Columbium (Niobium)		0.01–0.03
	Aluminum	0.010-0.040	0.020-0.050
	Nitrogen	0.015 max	0.015 max

^ANot applicable.

^B The steel shall be calcium treated for sulfide shape control.

TABLE 6 Grade 50S [345S] Chemical Requirements (Heat Analysis)

	<u>.</u>
Element	Composition, %
Carbon, max	0.23
Manganese	0.50 to 1.60 ^A
Silicon, max	0.40
Vanadium, max	0.15 ^B
Columbium, max	0.05 ^B
Phosphorus, max	0.035
Sulfur, max	0.045
Copper, max A/09/A/0	0.60 92
halog/standa_Nickel, max	0-40.45 ad16-b0794
Chromium, max	0.35
Molybdenum, max	0.15

 A Provided that the ratio of manganese to sulfur is not less than 20 to 1, the minimum limit for manganese for shapes with flange or leg thickness not exceeding 1 in. [25 mm] shall be 0.30 %.

^B The sum of columbium and vanadium shall not exceed 0.15 %.

TABLE 7 Relationship Between Impact Testing Temperature		
Zones and Minimum Service Temperature		

Zone	Minimum Service Temperature, °F [°C]
1	0 [-18]
2	below 0 to -30 [-18 to -34]
3	below -30 to -60 [-34 to -51]

3.1.1 *fracture critical member*—a main load-carrying tension member or tension component of a bending member whose failure would be expected to cause collapse of a structure or bridge without multiple, redundant load paths.

3.1.2 *main load-carrying member*—a steel member designed to carry primary design loads, including dead, live, impact, and other loads.

3.1.3 *non-fracture critical member*—a main load-carrying member whose failure would not be expected to cause collapse of a structure or bridge with multiple, redundant load paths.

3.1.4 non-tension component—a steel member that is not in tension under any design loading.

3.1.5 secondary member—a steel member used for aligning and bracing of main load-carrying members, or for attaching utilities, signs, or other items to them, but not to directly support primary design loads

3.1.6 *tension component* —a part or element of a fracture critical or non-fracture critical member that is in tension under various design loadings.