

Designation: D2937 - 10 D2937 - 17

Standard Test Method for Density of Soil in Place by the Drive-Cylinder Method¹

This standard is issued under the fixed designation D2937; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ε) indicates an editorial change since the last revision or reapproval.

This standard has been approved for use by agencies of the U.S. Department of Defense.

1. Scope*

- 1.1 This test method covers the determination of in-place density of soil by the drive-cylinder method. The test method involves obtaining a relatively an intact soil sample by driving a thin-walled cylinder and the subsequent activities—into the soil and conducting specific measurements and calculations for the determination of in-place density. When sampling or in-place density is required at depth, Test Method D1587 should be used.
- 1.2 This test method is not appropriate recommended for sampling organic or friable soils which can compress upon sampling, may compress during sampling. This test method may not be applicable for soft, organic, highly plastic, noncohesive, saturated or other soils which are easily deformed, compress during sampling, or which may not be retained in the drive cylinder sampler. This test may not be applicable with very hard natural soils andor heavily compacted soils which cannot that may not be easily penetrated bywith the drive sampler, soils of low plasticity which will not be readily retained in the cylinder, or soils which contain appreciable amounts of gravel (particles coarser than 4.75 mm (cylinder sampler. The use of this test method in soils containing an appreciable amount of 3/16 in.)). The presence of particles coarserparticles larger than 4.75 mm (3/16 in.) may introduce significant errors in density measurements by causing result in damage to the drive cylinder equipment. Soils containing particles larger than 4.75 mm (3/16 voids in.) may not yield valid results if voids are created along the wall of the cylinder during driving, and when coarse materials have to be dislodged by the trimming of the sample obtained by the cylinder.or if particles are dislodged from the sample ends during trimming.
- 1.3 This test method is limited to the procedures necessary for obtaining specimens suitable for determining the in-place density and water content of certain soils. The <u>procedures procedures, precautions</u>, and <u>precautions</u>requirements necessary for selecting locations <u>and for</u> obtaining intact samples, suitable for laboratory testing or otherwise determining engineering properties, is beyond the scope of this test method.
- 1.4 The values stated in SI units are to be regarded as standard. The inch-pound units given in parentheses are mathematical conversions, which are provided for information purposes only and are not considered standard.
- 1.4.1 It is common practice in the engineering/construction profession to concurrently use pounds to represent both a unit of mass (lbm) and a unit of force (lbf). This implicitly combines two separate systems of units; that is, the absolute system and the gravitational system. It is scientifically undesirable to combine the use of two separate sets of inch-pound units within a single standard. As stated, this standard includes the gravitational system of inch-pound units and does not use/present the slug unit for mass. However, the use of balances or scales recording pounds of mass (lbm) or the recording of density in lbm/ft³ shall not be regarded as nonconformance with this standard.
- 1.5 All observed and calculated values shall conform to the guidelines for significant digits and rounding established in Practice D6026, unless superseded by this standard.
- 1.5.1 The procedures used to specify how data are collected/recorded or calculated in this standard are regarded as the industry standard. In addition, they are representative of the significant digits that generally should be retained. The procedures used do not consider material variation, purpose for obtaining the data, special purpose studies, or any considerations for the user's objectives; and it is common practice to increase or reduce significant digits of reported data to be commensurate with these considerations. It is beyond the scope of this standard to consider significant digits used in analysis methods for engineering design.
- 1.6 This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.

¹ This test method is under the jurisdiction of ASTM Committee D18 on Soil and Rock and is the direct responsibility of Subcommittee D18.08 on Special and Construction Control Tests.

Current edition approved March 15, 2010 Feb. 1, 2017. Published April 2010 February 2017. Originally approved in 1971. Last previous edition approved in 20042010 as D2937 – 04:D2937 – 10. DOI: 10.1520/D2937-10:10.1520/D2937-17.



2. Referenced Documents

2.1 ASTM Standards:²

D653 Terminology Relating to Soil, Rock, and Contained Fluids

D698 Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³ (600 kN-m/m³))

D1557 Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³ (2,700 kN-m/m³))

D1587 Practice for Thin-Walled Tube Sampling of Fine-Grained Soils for Geotechnical Purposes

D2216 Test Methods for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass

D2488 Practice for Description and Identification of Soils (Visual-Manual Procedure)

D3740 Practice for Minimum Requirements for Agencies Engaged in Testing and/or Inspection of Soil and Rock as Used in Engineering Design and Construction

D4643 Test Method for Determination of Water Content of Soil and Rock by Microwave Oven Heating

D4753 Guide for Evaluating, Selecting, and Specifying Balances and Standard Masses for Use in Soil, Rock, and Construction Materials Testing

D4944 Test Method for Field Determination of Water (Moisture) Content of Soil by the Calcium Carbide Gas Pressure Tester

D4959 Test Method for Determination of Water Content of Soil By Direct Heating

D6026 Practice for Using Significant Digits in Geotechnical Data

3. Terminology

3.1 Definitions—All definitions are in accordance with For common terms found in this standard refer to Terminology D653.

4. Significance and Use

- 4.1 This test method can be used to determine the in-place density of soils which do not contain significant amounts of particles eoarserlarger than 4.75 mm ($\frac{3}{16}$ in.), and which can be readily retained in the drive cylinder. This test method may also be used to determine the in-place density of compacted soils used in construction of structural fill, highway embankments, or earth dams. When the in-place density is to be used as a basis for acceptance, the drive cylinder volumes must be as large as practical and not less than 850 cm³ (0.030 ft³).
- 4.2 This test method is not recommended for use in organic or friable soils. This test method may not be applicable for soft, highly plastic, noncohesive, saturated or other soils which are easily deformed, compress during sampling, or which may not be retained in the drive cylinder. The use of this test method in soils containing particles coarser than 4.75 mm (¾6 in.) may result in damage to the drive cylinder equipment. Soils containing particles coarser than 4.75 mm (¾6 in.) may not yield valid results if voids are created along the wall of cylinder during driving, or if particles are dislodged from the sample ends during trimming.
- 4.2 The general principles of this test method have been successfully used to obtain samples of some various field compacted fine-grained soils having a maximum particle size of 4.75 mm (3/16 in.) for purposes other than density determinations, such as the testing for engineering properties.

Note 1—Notwithstanding the statements on precision and bias contained in this standard: The precision of this test method is dependent on the competence of the personnel performing it and the suitability of the equipment and facilities used. Agencies which meet the criteria of Practice D3740 are generally considered capable of competent and objective testing. Users of this method are cautioned that compliance with Practice D3740 does not in itself assure reliable testing. Reliable testing depends on many factors; Practice D3740 provides a means of evaluating some of those factors.

5. Apparatus

- 5.1 *Drive Cylinders*, of approximately $\frac{102}{100}$ to 152 mm (4.00 to 6.00 in.) diameter or larger diameter. Larger sizes may be used if desired or required. Typical details of drive cylinders with outside diameters of $\frac{102}{100}$ mm (4.00 in.) are shown in Fig. 1 (see also Table 1). Drive cylinders of other diameters will require proportional changes in the drive-cylinder tube and drive-head dimensions. The volume of the cylinders with the dimensions shown in Fig. 1 is approximately 940 cm³ (0.033 ft³). The apparatus shown in Fig. 1 is of a design suitable for use at or near the surface.
- 5.1.1 When the in-place density is to be used as a basis for acceptance of compacted fill, the <u>drive</u> cylinders shall be as large as practical to <u>minimize</u> reduce the effects of errors and shall in no case be smaller be equal to or greater than 850 cm³ (0.030 ft³).
- 5.1.2 The number of <u>drive</u> cylinders required <u>depends</u> on the number of samples to be taken and the anticipated rapidity by which the cylinders can be returned to service after <u>weighing</u>, <u>eleaning</u>, <u>ete.processing</u>.
- 5.1.3 The cylinders shown in Fig. 1 meet the clearance ratio, wall thickness and area-ratio requirements as set forth by Hvorslev³ for drive cylinder samplers, and should preferably shall not exceed 10 to 15 %, as defined by the following:

$$A_r = [(Dw^2 - De^2)/De^2] \times 100 \tag{1}$$

² For referenced ASTM standards, visit the ASTM website, www.astm.org, or contact ASTM Customer Service at service@astm.org. For *Annual Book of ASTM Standards* volume information, refer to the standard's Document Summary page on the ASTM website.

³ Hyorslev, M. J., "Surface Exploration and Sampling of Soils for Engineering Purposes," Engineering Foundation, 345 E. 47th St., New York, NY 10017.

DRIVE HEAD DRIVE HAMMER VENT-19 mm DRILL HAND GRIP WALL THICKNESS (VARIABLE) - 101 DRIVE CYLINDER STEM —— 19 mm COLD ROLLED STEEL WALL THICKNESS SLIDING WEIGHT APPROX. MASS 4.5 kg VOLUME 0.94 L 1096 127 15 deg. CHAMFER 865 HAND HOLD 115

All length dimensions are in millimeters.

FIG. 1 Typical Design for a Surface Soil Sampler

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TABLE 1 Dimensional Equivalent for Fig. 1

Trible 1 Billionological Equivalent for Fig. 1				
mm	in.	mm	in.	
2	5/64	103	4 1/16	
4	5/32	115	41/2	
5	3/16	127	5.00	
19	3/4	155	6.00	
44	13/4	212	8.00	
76	3.0	865	36.00	
98	37/8	1096	45.00	
102	4.00			
100	4.00			

where:

 A_r = area ratio, %,

 \overrightarrow{Dw} = maximum external diameter of the drive cylinder, and

De = effective (minimum) internal diameter of the drive sampler at the cutting edge after swaging.

5.1.4 Except for very short <u>drive cylinder</u> samplers with no clearance, the inside clearance ratio of the <u>drive cylinders</u> should shall be from 0.5 to 3.0 %, with increasing ratios as the plasticity increases in the soil being sampled. Inside clearance ratio is defined by the following:

$$C_{\rm r} = \frac{Di - De}{De} \times 100 \tag{2}$$

where:

 $C_{\rm r}$ = inside clearance ratio, %



De = effective (minimum) internal diameter of the sampler at the cutting edge after swaging, and

Di = internal diameter of the sampler.

- 5.1.5 Cylinders Drive cylinders of other diameters should shall conform to these requirements.
- 5.2 *Drive Head*—The typical details of the drive <u>headshead</u> and appurtenances are shown in Fig. 1. The drive head has a sliding weight for driving the cylinder.
- 5.3 Straightedge—steel, Steel, approximately 3 mm (1/8 in.) by 38 mm (11/2 in.) by 305 mm (12.0 in.) with one edge sharpened at approximately a 45° angle for trimming the ends of the sample flush with the cylinder.
- 5.4 *Shovel*—Any one of several types of shovels or spades is satisfactory in shallow sampling for digging the cylinders out after they have been driven into the soil.
- 5.5 <u>Balances—Balance—A</u> balance having a minimum capacity of 10 kg (22 lbs) and meeting the requirements of Specification D4753 for a balance of 1 g (0.002 lbs) readability is required for the cylinders shown in Fig. 1. Larger cylinders will require a balance of 25 kg (55 lbs) capacity with readability of 1 gm (0.002 lbs).
- 5.6 *Drying Equipment*—Equipment or ovens, or both, to determine dry specimens, facilitating the determination of water (moisture) content in compliance accordance with Test Methods D2216, D4643, D4944, or D4959.
- 5.7 Miscellaneous Equipment—Brushes, sledgehammers, plastic bags, metal cans with lids, or other suitable containers for retaining the drive cylinder and sample until the determination of moist mass and drying, spoons, inside/outside vernier caliper, or the equivalent water content can be determined. Spoons, inside/outside caliper, or equivalent, accurate to 0.25 mm (0.01 in.) for calibration, gloves, and safety glasses.calibration.
 - 5.8 Safety Equipment—Gloves and safety glasses. Steel-toed shoes or boots if required by agency.

6. Procedure

- 6.1 Brush all loose particles from the surface. For near-surface sampling (not more than 1 m (3 ft) in depth), sample through a hole bored with an auger or dug by a shovel from which loosened material has been removed. Obtain a fairly level ground surface before the cylinder is The surface where the cylinder initially is placed should be fairly level prior to the cylinder being driven. Depending on the soil texture type and moisture, moisture condition, the surface may be prepared utilizing a bulldozer blade or other heavy equipment blades provided providing the sample area and vicinity are not deformed, compressed, torn, or otherwise disturbed.
- 6.2 Assemble the cylinder and drive apparatus with the sharpened edge on the surface to be sampled. Drive the cylinder by raising the drop hammer and allowing it to fall, or alternatively by applying a uniform force via a jack or similar device, while keeping the drive rod steady and in a vertical position. Continue driving until the top of the cylinder is approximately 13 mm (1/2(1/2) in.) below the original surface as shown in Fig. 2. Overdriving may result in deforming or compressing the sample and may provide erroneous influence the test results. Care shouldshall be takenexercised to prevent overdriving, particularly when sampling below the surface. If overdriving occurs or is suspected, the sample shouldshall be discarded and the soil resampled. Remove the drive head and digremove the cylinder from the ground with a shovel; dig the soil from around the sides of the cylinder, undercutting several inches below the bottom of the cylinder before lifting the cylinder out. out from the ground. When sampling near the surface, more soil may haveneed to be dugremoved from around the sides of the cylinder to properly undercut the cylinder.
- 6.3 After the cylinder has been removed from the ground, remove any excess soil from the sides of the <u>drive</u> cylinder. Using the straightedge, trim the ends of the sample flush and plane with the ends of the cylinder. <u>Patch with loose soil any voids that may have been created from the trimming process</u>. A satisfactory sample is <u>composed consists</u> of an intact soil <u>sample</u> and shall

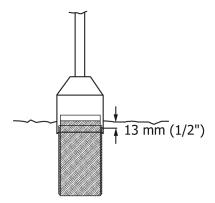


FIG. 2 Relationship of Driven Cylinder to Ground Surface

not contain rocks, roots, or other foreign material. If the <u>drive</u> cylinder is not full or does not properly represent the in-situ soil, discard <u>the soil</u> and <u>takeobtain</u> another sample. If the <u>drive</u> cylinder is deformed or otherwise damaged <u>while as a result of driving</u> it into or removing it from the ground, <u>diseard and repair</u> or replace the <u>drive</u> cylinder. Immediately determine the mass and water content of the sample or place the drive cylinder and sample in a <u>container moisture proof container</u>, which will prevent soil or water loss until mass and water determinations can be made.

- 6.4 Record the mass of the drive cylinder and soil sample to the nearest 1 g (0.002 lbm).
- 6.5 Remove the soil from the cylinder. Obtain a representative specimen for water content determination. Specimens for determining water content are to be as large as practical but in no case smallershall not be less than 100 g (0.200 lbs) and selected to represent all the material from the cylinder. Determine the water content of the soil in accordance with Test Methods D2216, D4643, D4944, or D4959.
 - 6.6 Classify the soil in general accordance with Practice D2488 or other standard means of soil classification.

7. Calculation

- 7.1 The density of the soil is expressed as the mass of the soil divided by the volume of soil, and is usually reported in grams per cubic centimeter (g/cm³) or pounds per cubic foot (lb/ft³).
 - 7.2 Calculate the wet density, ρ_{wet} , of the drive-cylinder sample in g/cm³ as follows:

$$\rho_{wet} = \frac{(M_1 - M_2)}{V} \tag{3}$$

where:

 M_I = mass of the cylinder and wet soil sample, g

 M_2 = mass of the cylinder, g, and

V = volume of the drive cylinder, cm³

7.3 Calculate the in-place dry density, ρ_d , of the soil in g/cm³ as follows:

$$(https://st^{\rho_d} = \frac{\rho_{wet}}{(1 + (w/100))} ds. iteh. 2i)$$
(4)

where:

 ρ_d = in-place dry density, g/cm³ **Document Preview**

 ρ_{wet} = in-place wet density, g/cm³, and

w =water content, %, dry mass basis.

7.3.1 Dry Unit Weight:

ASTM D2937-17

https://standards.iteh.ai/catalog/standards/sist $\gamma_d = K_1 \times \rho_d (in kN/m^3)$ -4100-a41a-24fd807ed92a/astm-d2937-17

or

$$\gamma_d = K_2 \times \rho_d \left(in \, lbf/ft^3 \right) \tag{6}$$

where:

 ρ_d = in-place dry density, g/cm³,

 $K_I = 9.81$ for density in g/cm³, and

 $K_2 = 62.4$ for density in lb/ft³.

Note 2—It may be desired to express the in-place density as a percentage of some other density, for example, the laboratory maximum density, determined in accordance with Test Methods D698 or D1557. This relation can be determined by dividing the in-place density by the maximum density and multiplying by 100.

7.4 If desired calculate the percent of a selected dry density in percent as follows:

$$P = (\rho_d / \rho_t) \times 100 \tag{7}$$

where:

P =percent of selected dry density,

 $\rho_d = \frac{\text{dry density of drive cylinder sample in g/cm}^3 \text{ or lb/ft}^3$, and

 $\rho_t = \text{selected dry density in g/cm}^3 \text{ or } \frac{\text{lb/ft}^3}{\text{lb}}$

Note 2—It may be desired to express the in-place density as a percentage of the laboratory maximum density, determined in accordance with Test Methods D698 or D1557.

8. Report: Test Data Sheet(s)/Form(s)

- 8.1 Record as a minimum the following general information (data):
- 8.1.1 Project No., Location, Date Test(s) Performed, Tests Performed By, Performed.
- 8.1.2 Person Test(s) Performed By.

- 8.1.3 Sample/specimen identifying information, such as, Test No., depth below surface or elevation (cm).
- 8.2 Record as a minimum the following test specimen data:
- 8.2.1 The mass and dimensions (length and diameter) and volume of the drive cylinder, to either three or four significant digits, see Annex A1.
- 8.2.2 The water content (nearest 0.1 percent) and to the nearest 0.1 percent, test method used, and dry unit weight (three to three or four significant digits, see 7.2Section, 7.27.3, and 7.3.1) of the test sample.
 - 8.2.3 Visual description of the soil sample, and
 - 8.2.4 Comments on soil sample disturbance.
- 8.3 If the in-place dry density or unit weight is expressed as a percentage of another value, or used as a basis for acceptance of compacted fill, include the following:
 - 8.3.1 The comparative dry density or unit weight value and water content used,
 - 8.3.2 The method used to determine the comparative values,
 - 8.3.3 The comparative percentage of the in-place material to the comparison value, and
 - 8.3.4 The in-place dry density as a percent of a selected dry density if so desired.
 - 8.3.5 The acceptance criteria applicable to the test.

9. Precision and Bias

- 9.1 *Precision*—Test data on precision are not presented due to the nature of this method. It is either not feasible or too costly at this time to have ten or more agencies participate in an in-situ testing program at a given site.
- 9.1.1 Limited past studies running repetitive adjacent tests on the same soil using undersize cylinders having inside diameters 7.3 cm of 73 mm (2½ in.), have indicated standard deviations of 32 kg/m³ (2.00 lb/ft³) to 46.4 kg/m³ (2.90 lb/ft³) for soils with a compacted wet density ranging from 2022 kg/cm³ (126.2 lb/ft³) to 2154 kg/m³ (134.5 lb/ft³).4
- 9.1.2 In another study, running repetitive adjacent tests on the same soil using a 13.0 cm 130 mm (51/8 in.) inside diameter cylinder, a standard deviation of 31 kg/m³ (1.93 lb/ft³) was obtained for soil with a compacted wet density of about 2000 kg/m³ (125 lb/ft³). In general, a lower standard deviation should be expected with a larger diameter drive cylinder.
 - 9.1.3 Subcommittee D18.08 is seeking pertinent data from users of this test method on precision.
 - 9.2 Bias—There is no accepted reference value for this test method, therefore, bias cannot be determined.

10. Keywords

10.1 compaction control; density testing; drive cylinder; drive cylinder test; field density; in-place density; plug sampler; quality control; subsurface sampler; surface sampler; unit weight

ASTM D2937-17

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(Mandatory Information)

A1. CALIBRATION OF DRIVE CYLINDER TUBE

A1.1 Scope

- A1.1.1 This annex describes the procedure for determining the volume of a drive cylinder tube.
- A1.1.2 The volume is determined by linear measurements method.

A1.2 Apparatus

- A1.2.1 In addition to the apparatus listed in Section 5 the following items are required:
- A1.2.1.1 Vernier Digital or Dial Caliper—Having a measuring range of at least 0 to 150 mm (0 to 6.0 in.) and readable to at least 0.02 mm (0.001 in.).

⁴ Noorany, I., Gardener, W.S., Corley, D.J., and Brown, J.L., "Variability in Field Density Tests," Constructing and Controlling Compaction of Earth Fills, ASTM STP 1384 March 2000

⁵ McCook, D. K., and Shanklin, D., "Nuclear Density Testing and Comparisons with Sand Cone and Calibrated Cylinder Methods," *Constructing and Controlling Compaction of Earth Fills*, ASTM STP 1384, March 2000.